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Evaluation of Landslide Hazard Assessment Models at Regional Scale – *(SciNet NatHazPrev Project)*

Democritus University of Thrace (P1)
Department of Civil Engineering
Geotechnical Division

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Activity 1.7:

Evaluation of existing Landslide Hazard assessment models in terms of **scientific soundness**, **data demands**, **results credibility**

Activity 1.11:

Development / Modification / Adaptation of existing models used to assess landslide hazard, **based on local conditions**.

Key parameters:

- Successful assessment of the areas prone to slide
- **Data to be provided**
- Complexity of the model

Scale: Regional (1:250,000 to 1:25,000)

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1. Landslide Susceptibility (static conditions) *according to FEMA method*

- 1. Geology (lithology per geologic group)**
- 2. Slope angle**
- 3. Water table**

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Table 1. Landslide susceptibility of geologic groups under static conditions (HazUS MH, Chapter 4 – PESH.) – FEMA method

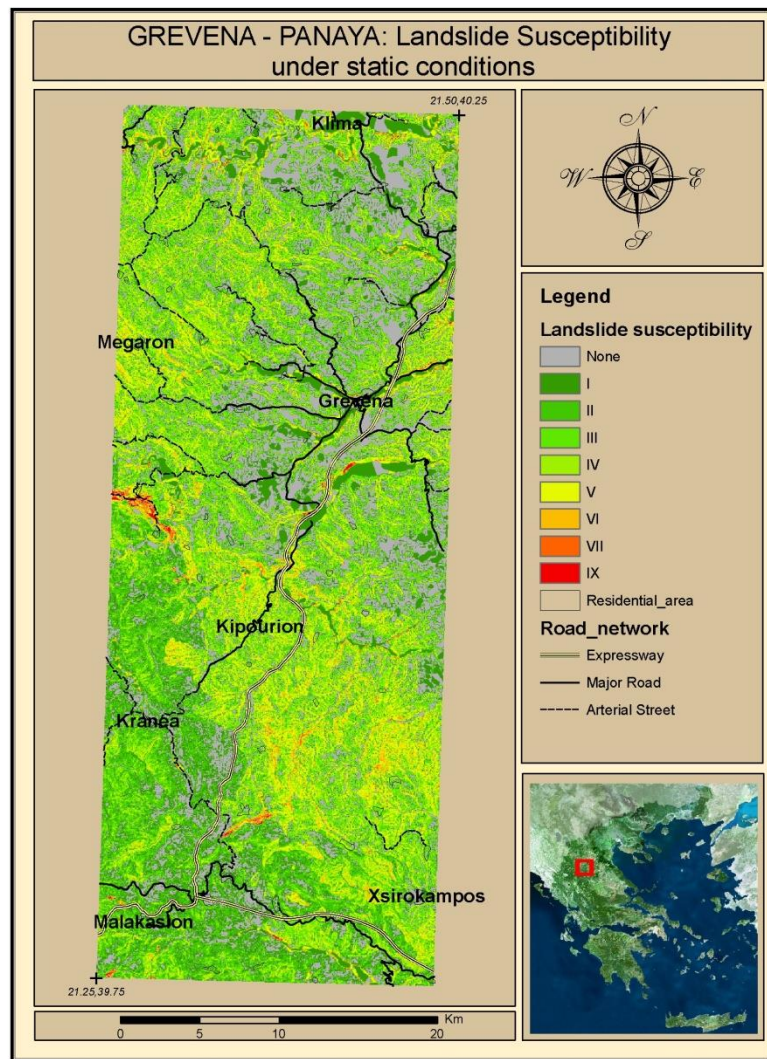
Geologic Group		Slope Angle, degrees					
		0-10	10-15	15-20	20-30	30-40	>40
(a) DRY (groundwater below level of sliding)							
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 300$ psf, $\phi' = 35^\circ$)	None	None	I	II	IV	VI
B	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0$, $\phi' = 35^\circ$)	None	III	IV	V	VI	VII
C	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, $c' = 0$, $\phi' = 20^\circ$)	V	VI	VII	IX	IX	IX
(b) WET (groundwater level at ground surface)							
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 300$ psf, $\phi' = 35^\circ$)	None	III	VI	VII	VIII	VIII
B	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0$, $\phi' = 35^\circ$)	V	VIII	IX	IX	IX	X
C	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, $c' = 0$, $\phi' = 20^\circ$)	VII	IX	X	X	X	X

□ Correspondence at a susceptibility category of Table 1, is based on a triple criterion control:

- **classification of geologic group**
- **slope angle**
- **hydraulic conditions**

□ Arbitrary scale from **I to X level**, the former (I) being the less susceptible and the latter (X) being the most susceptible

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Development of an Information
System for Natural Risk
Management in the Mediterranean
(SyNaRMa Project, ITSAK)

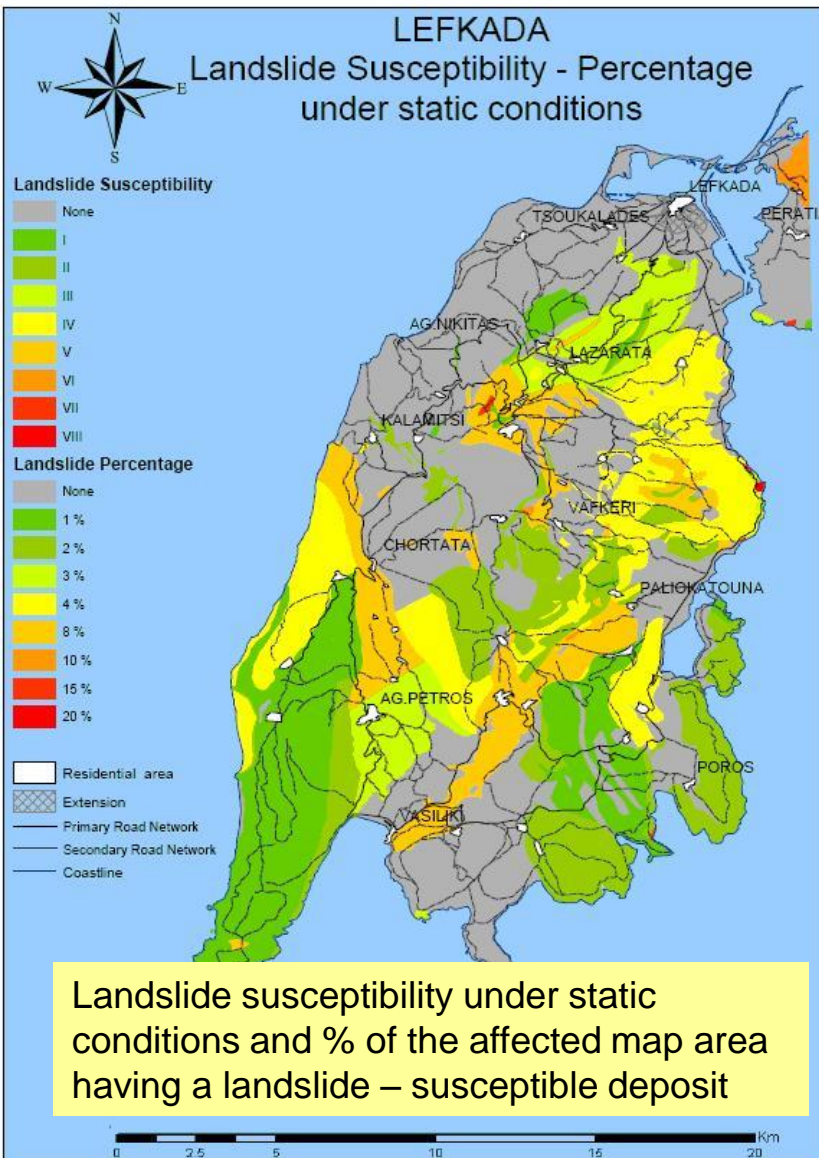
Landslide susceptibility **under static conditions** according to FEMA method
(based on geologic group and slope angle)

That was a successful example!

Scale 1:50,000



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Landslide susceptibility under static conditions (based on geologic group and slope angle and hydraulic conditions)

That was NOT a successful example!

The west part of the island, even though dominated, along the coastline, by mountains with high and rather steep natural slopes, **landslide susceptibility is practical null or very low**, whereas the flat or smoothed eastern part of the island, dominated by alluvial deposits, presents higher susceptibility.

The reasons for this “paradox”?

- susceptibility based on natural slopes, whilst failures occurred mainly in cut slopes and downstream road embankment slopes
- incompleteness of the model

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Table 1 Examples of hazard descriptors for dealing with potential landslides at different scales of work

Scale of work	Runout	$I(M)/F^a$	Hazard descriptor
National <1:250,000	Not included	Not considered	No. of landslides/ administrative unit/year
Regional 1:250,000–1:25,000	Usually not included	Often a fixed (constant) magnitude value	No. of landslides/ km ² /year
Local 1:25,000–1:5,000	Included	Spatially distributed magnitude (intensity)	Annual probability of occurrence (or return period) of a given magnitude or intensity
Site-specific >1:5,000	Included	Spatially distributed intensity	Annual probability of occurrence (or return period) of a given intensity

^a Intensity (magnitude)/frequency

The current practice in Europe (Corominas et al., 2010) shows that the scale of the **landslide zoning maps** required by state or local authorities **varies significantly from country to country**, depending on *the coverage, input data and methods used, as well as, the information provided (qualitative or quantitative)*

Typical areas to be examined in **regional scale** usually in early phases of regional development projects or for engineers evaluating possible constraints due to ground instabilities / failures during large engineering projects.

**Corominas et al., 2013;
based on SafeLand project**

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Susceptibility and Hazard maps are usually based on the following assumptions:

- *homogeneous geological conditions*
- *all slopes have the same probability of failure*
- *exact location of slope failure NOT required*
- *all landslides are of similar size*
- *runout distance is NOT calculated; NOR spatial distribution and intensity*

This could be modified! **HOW??**

By introducing **STRUCTURAL** information

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Predisposing factors play an important role in **landslide susceptibility and hazard analysis**:

- **topographic information and its derivatives** (need for high-resolution DEMs)
- **geologic maps** focusing traditionally into lithological & stratigraphical subdivision should be converted into an engineering geological classification with emphasis on Quaternary sediments and rock texture/structure & rock mass strength
- **structural information** is important for landslide hazard assessments; attempts to incorporate dip & dip direction based on field measurements depend strongly on the number of measurements and complexity of structure
- **soil properties** in the use of physically based slope stability models for LHA (Landslide Hazard Assessment) are key parameters, especially for shallow depth failures. **Soil depth, defined as the depth from the surface to a consolidated material**
- **spatial variability** is also a crucial parameter, often ignored in landslide modelling
- **Soil thickness** can be modeled throughout physical based methods that model rates of **weathering, denudation and accumulation**

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- **Physically based landslide susceptibility** assessment methods are based on the modeling of slope failure processes

- Applicable over large areas if geological & geomorphological conditions are fairly homogeneous and landslide types relatively simple
- Applicable to areas with **incomplete landslide inventories**

- Most of them apply the **infinite slope model**, therefore they are applicable in the case of **shallow landslides**
- They account for different triggering parameters: **rainfall** and transient groundwater response or to the effects of **earthquake excitation**

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Main advantages:

- They can be easily implemented in GIS framework
- Results are more concrete & consistent compared to other approaches
- Higher predictive capability and most suitable to quantify the influence of individual parameters contributing to shallow landslide initiation
- Based on slope stability models, they allow the calculation of quantitative values of stability (safety factor, probability of failure)

Main drawbacks:

- Parameterisation can be a difficult task; access to critical parameters (soil depths, transient slope hydrological processes & temporal changes in hydraulic properties)
- Degree of simplification encountered / need for large amounts of reliable input data

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2. Landslide Susceptibility (seismic conditions) *according to FEMA method*

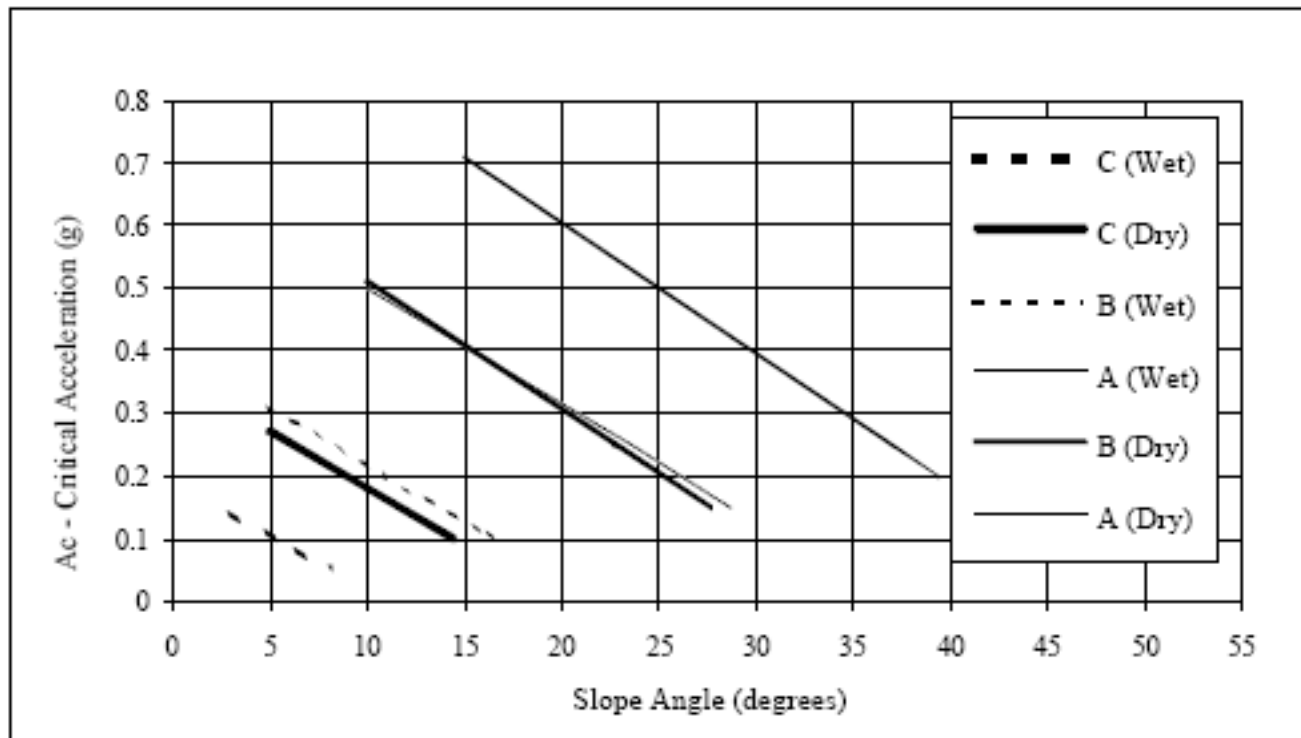
- 1. Geology (lithology per geologic group)**
- 2. Slope angle**
- 3. Water table**

AND

- 4. Critical Acceleration**

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Critical acceleration (a_c), is a crucial parameter that is added to the aforementioned, being a complex function of slope, geology, steepness, groundwater table, type of landsliding and history of previous slope performance. The relationship of Wilson and Keefer (1985) is utilized in the method adopted herein.



**Critical Acceleration as a Function of Geologic Group and Slope Angle
(Wilson and Keefer, 1985).**

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Lower Bounds for Slope Angles and Critical Accelerations for Landsliding Susceptibility

Group	Slope Angle, degrees		Critical Acceleration (g)	
	Dry Conditions	Wet Conditions	Dry Conditions	Wet Conditions
A	15	10	0.20	0.15
B	10	5	0.15	0.10
C	5	3	0.10	0.05

Critical Accelerations (a_c) for Susceptibility Categories

Susceptibility Category	None	I	II	III	IV	V	VI	VII	VIII	IX	X
Critical Accelerations (g)	None	0.60	0.50	0.40	0.35	0.30	0.25	0.20	0.15	0.10	0.05

Percentage of Map Area Having a Landslide-Susceptible Deposit

Susceptibility Category	None	I	II	III	IV	V	VI	VII	VIII	IX	X
Map Area	0.00	0.01	0.02	0.03	0.05	0.08	0.10	0.15	0.20	0.25	0.30

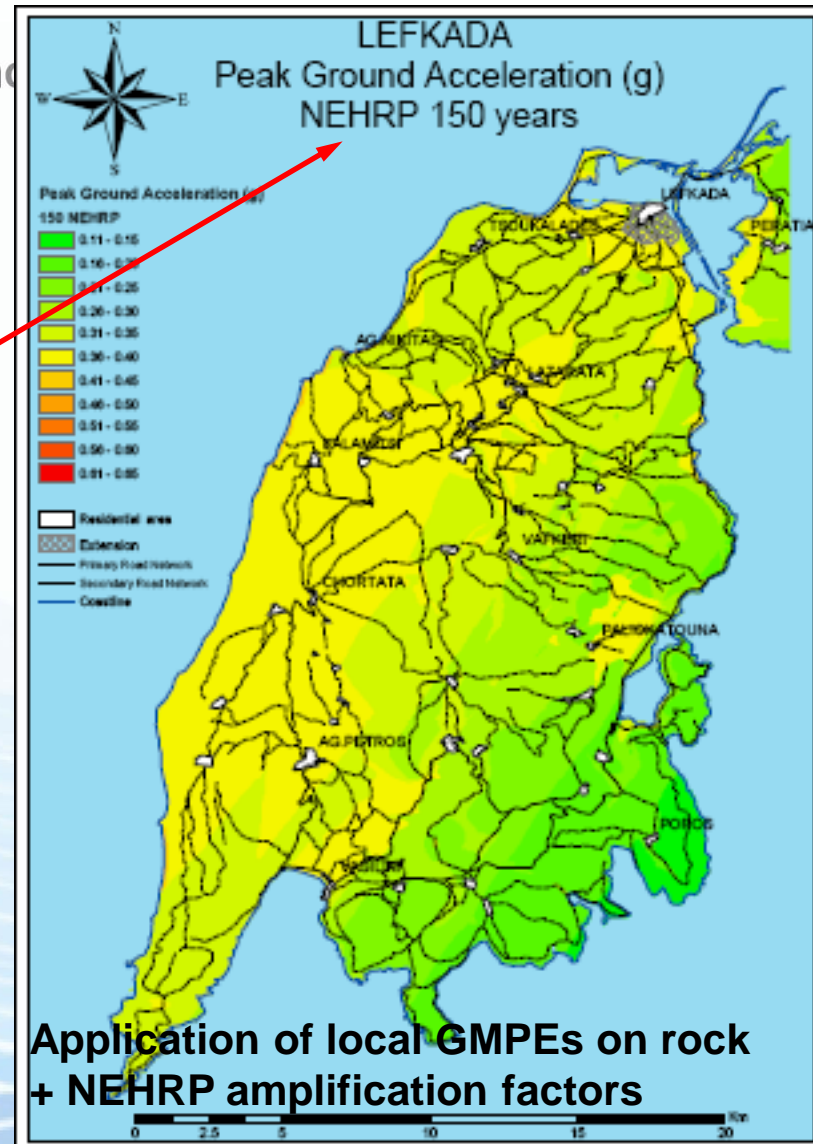
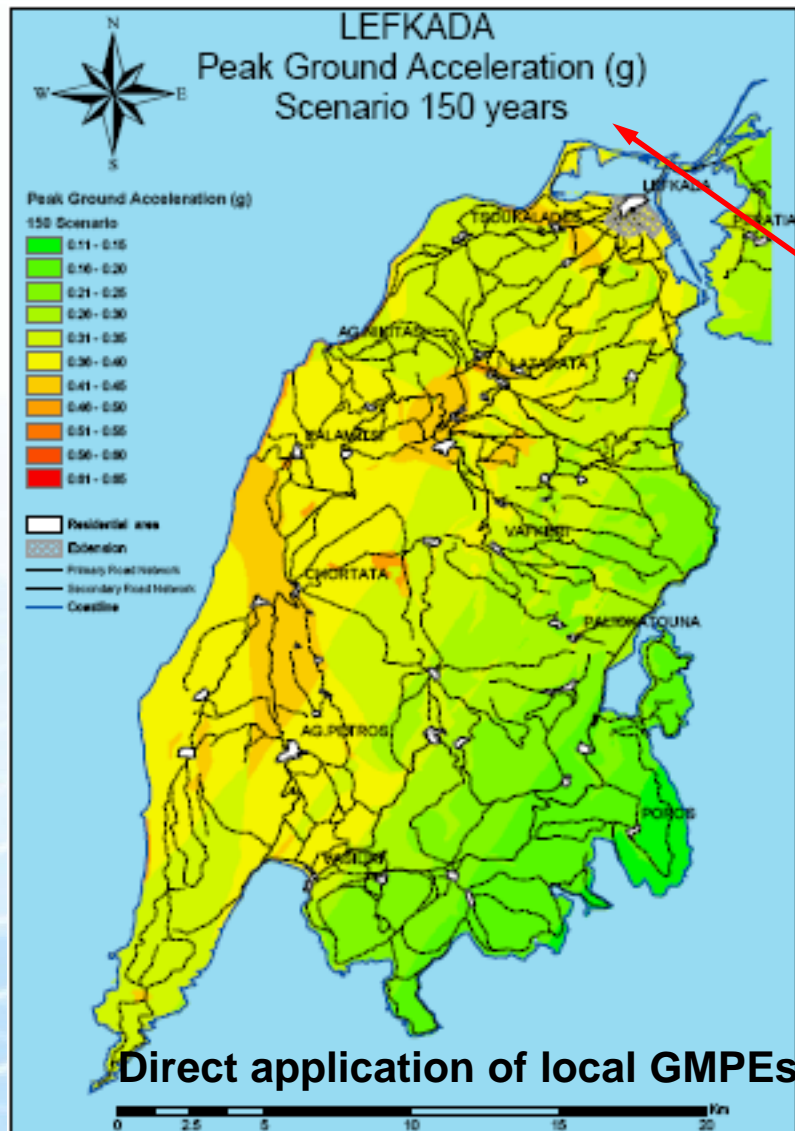


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PGA spatial
distribution
from
LEFKADA
island
earthquake
M6.2, 14-8-
2003



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“Shallow” landslides susceptibility under seismic conditions

“Shallow” landslide susceptibility to earthquake-induced displacements, as specified by the index A_c/PGA for a mean return period of a seismic event or a discrete seismic event (M6.2, 2003; Lefkada island)

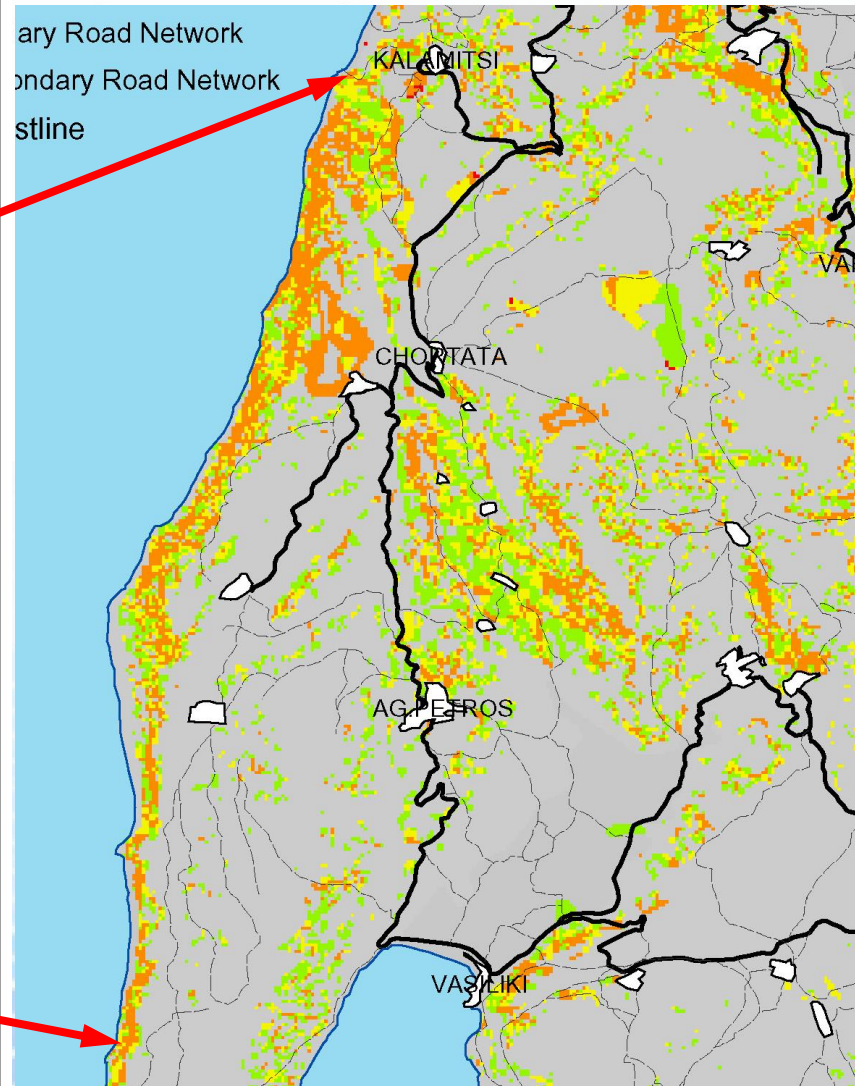
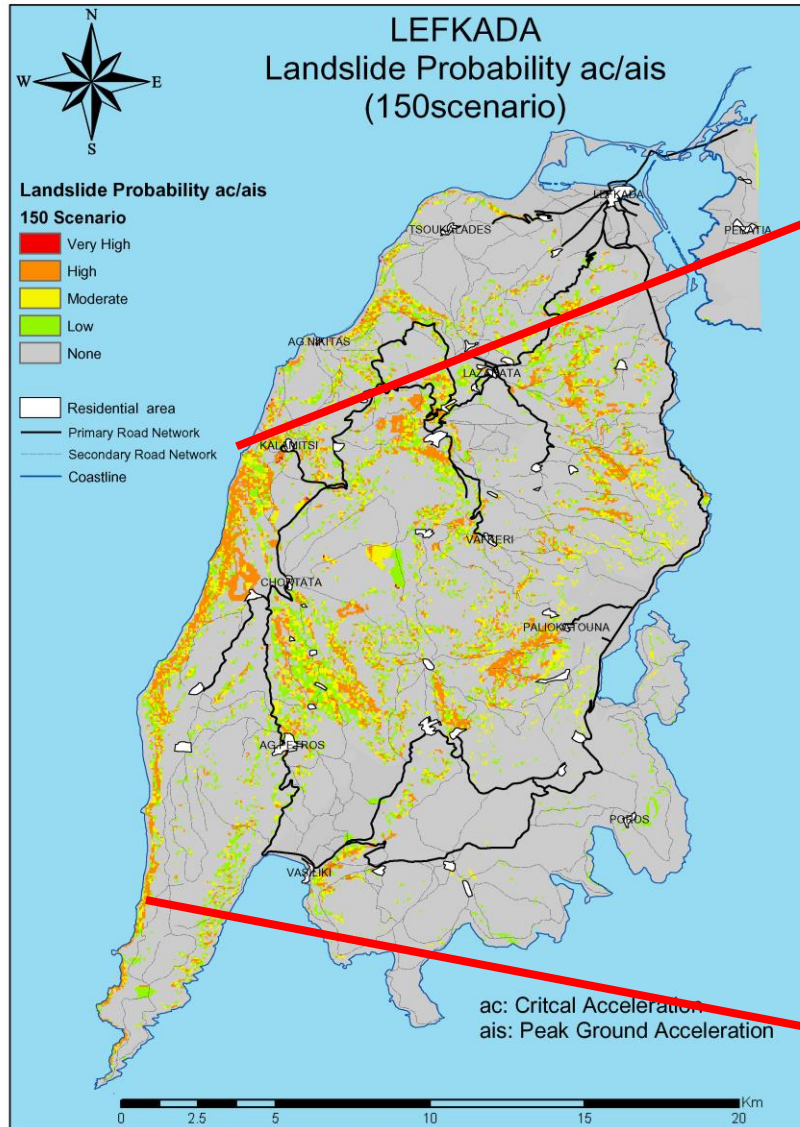
On what criterion?

Index A_c/PGA and a
“subjective” categorization

- ☐ Very high: < 0.3
- ☐ High: $0.3 - 0.6$
- ☐ Moderate: $0.6 - 0.8$
- ☐ Low: $0.8 - 1.0$
- ☐ Very Low: $1.0 - 3.0$
- ☐ None: > 3.0



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3. Landslide Hazard Assessment

(under static / hydraulic and seismic conditions)

Three (3) approaches for landslide hazard assessment were tested:

- ☐ HazUS method proposed by FEMA adapted to Hellenic data (**triggering: earthquake**),
- ☐ Newmark modified method (**triggering: earthquake**), and
- ☐ the static factor of safety F_s computation method (**triggering: static/hydraulic conditions**).

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3. Landslide Hazard (seismic conditions) according to FEMA method (a)

Assessment of Permanent Ground Displacements (PGD) of
“shallow” landslides

$$E[PGD] = E[d/A_{is}] * A_{is} * n$$

$E[d/A_{is}]$:

A_{is} : induced acceleration (g) – same with PGA

n : number of cycles

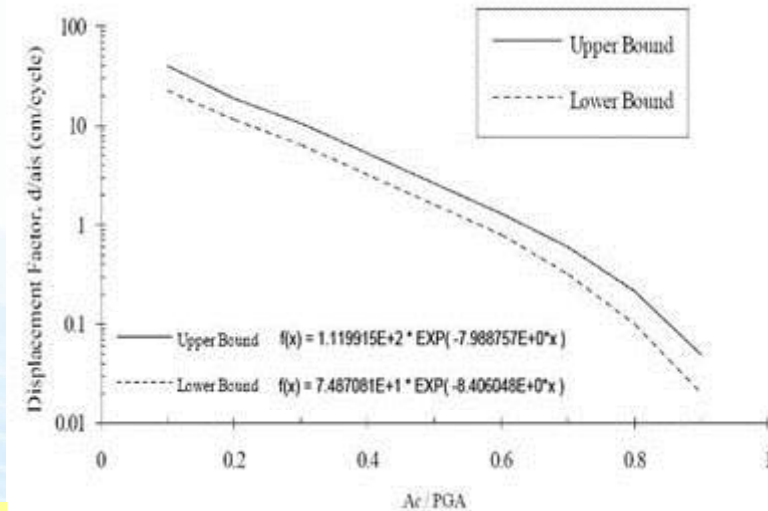
$E[d/A_{is}]$: expected displacement factor

$$n = 0.3419M_w^3 - 5.5214M_w^2 + 33.6154M_w - 70.7692$$

Local GMPEs (Skarlatoudis et al., 2003)

$$\log PGA = 1.07 + 0.45M - 1.35 \times \ln(R + 6) + 0.09F + 0.06S \pm 0.286$$

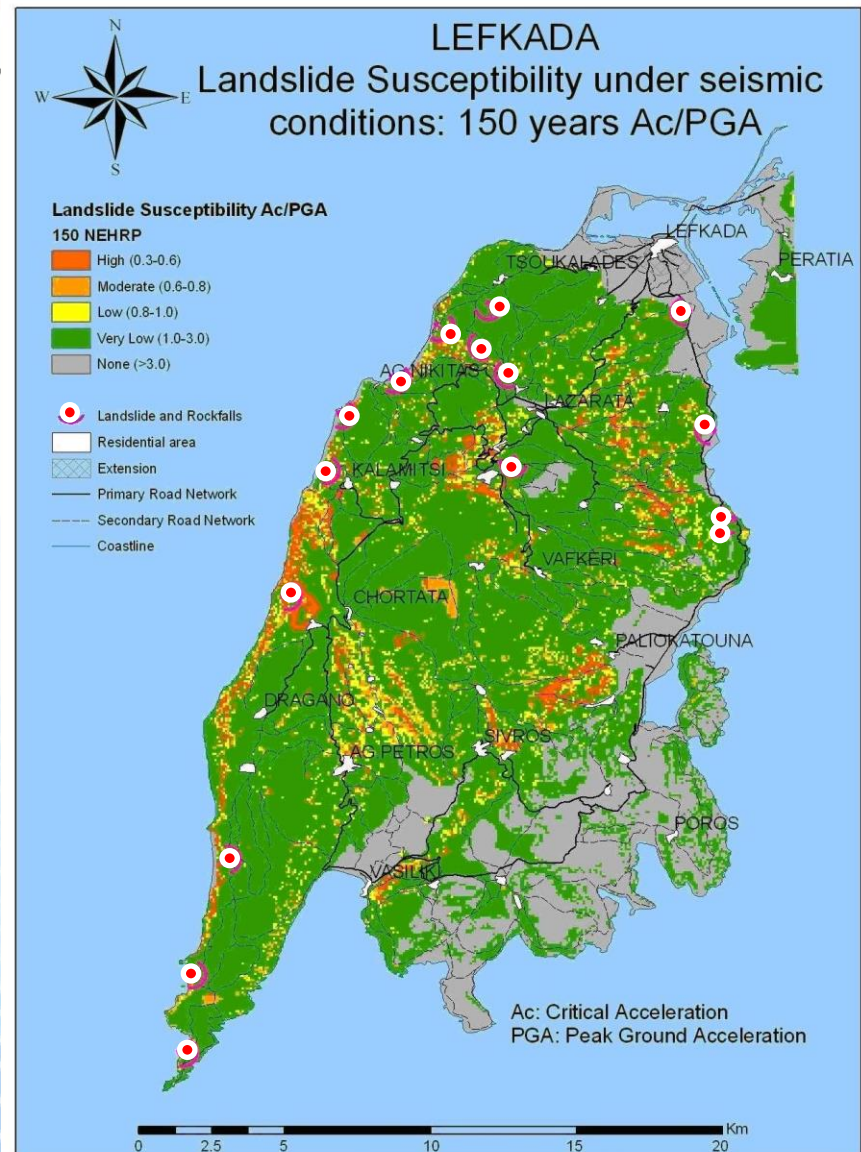
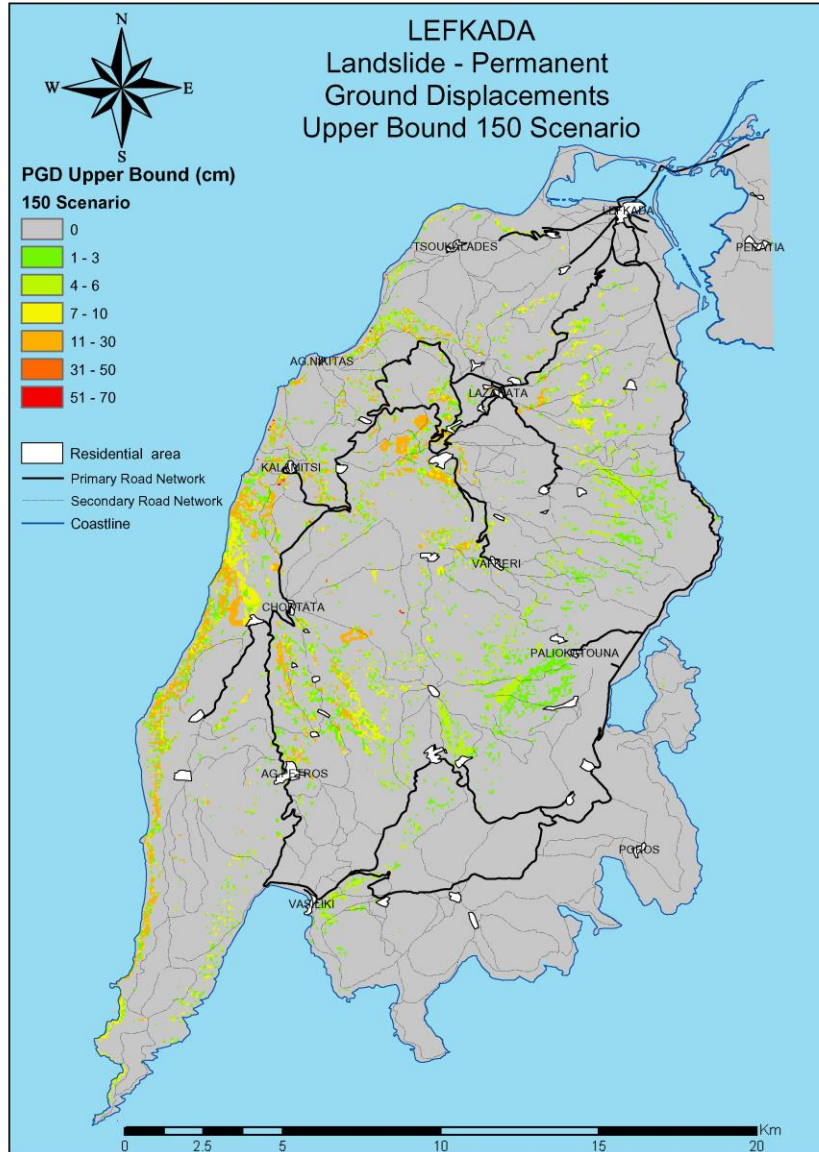
$$\log PGA = 0.86 + 0.45M - 1.27 \times \ln(R^2 + h^2)^{\frac{1}{2}} + 0.10F + 0.06S \pm 0.286$$



A_c / PGA



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3. Landslide Hazard (seismic conditions) *according to modified Newmark method (b)*

$$\log D_n = 1.521 \log I_0 - 1.993 \log A_c - 1.546 \quad \text{For slopes } > 10^\circ$$

D_n : Newmark displacements (cm)

I_0 : Arias Intensity (m/sec)

A_c : critical acceleration

$$I_0 = \frac{\pi}{2g} \int_0^\infty [a(t)]^2 dt \quad \left\{ \begin{array}{l} I_0 \text{ Arias Intensity (1970),} \\ g \text{ ground acceleration} \\ a(t) \text{ time series acceleration} \end{array} \right.$$

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*Landslide hazard
estimation methods
using GIS: method
evaluation and
implementation in
Magnesia prefecture*

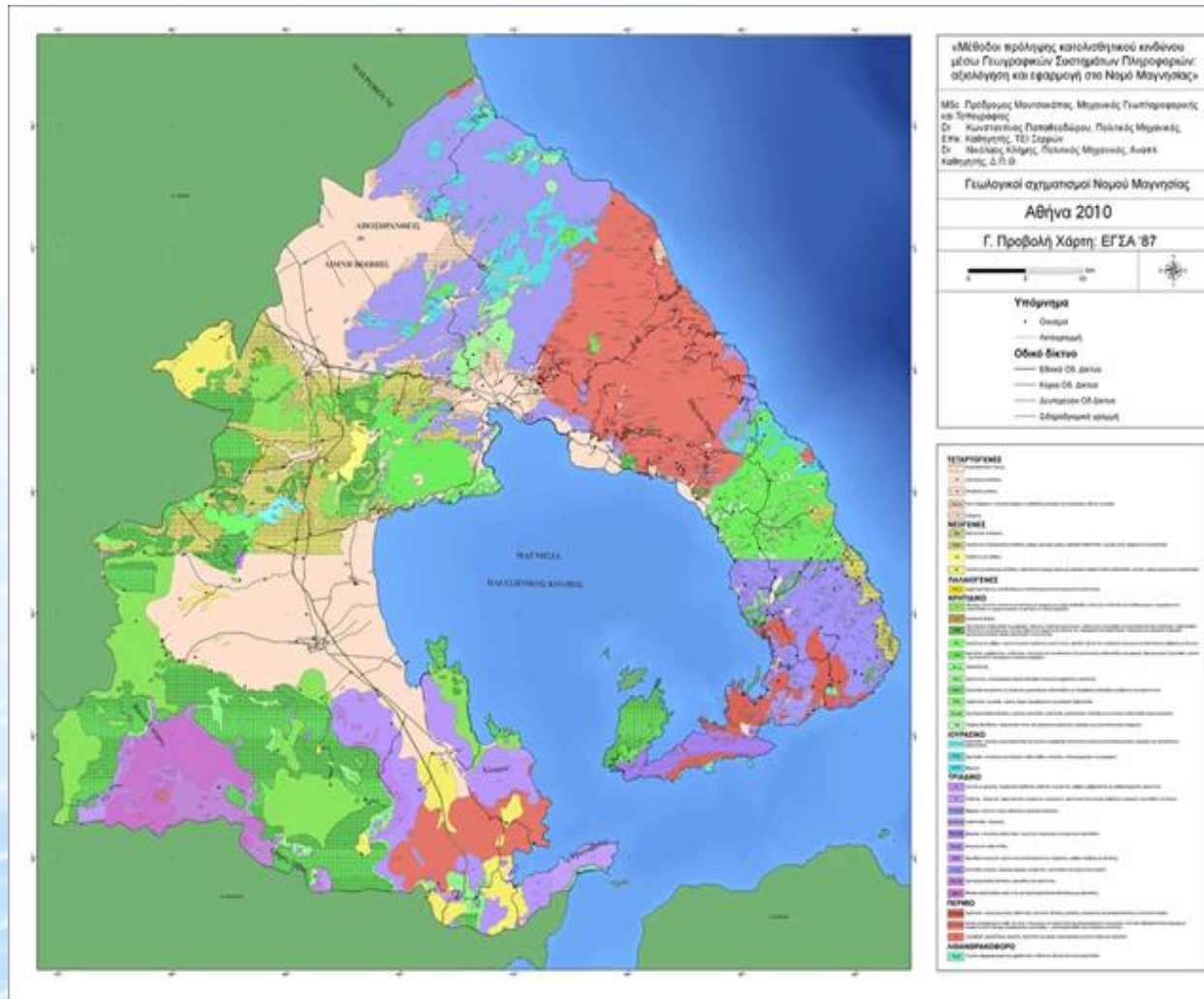


Figure 1. Geologic map of the Prefecture of Magnesia



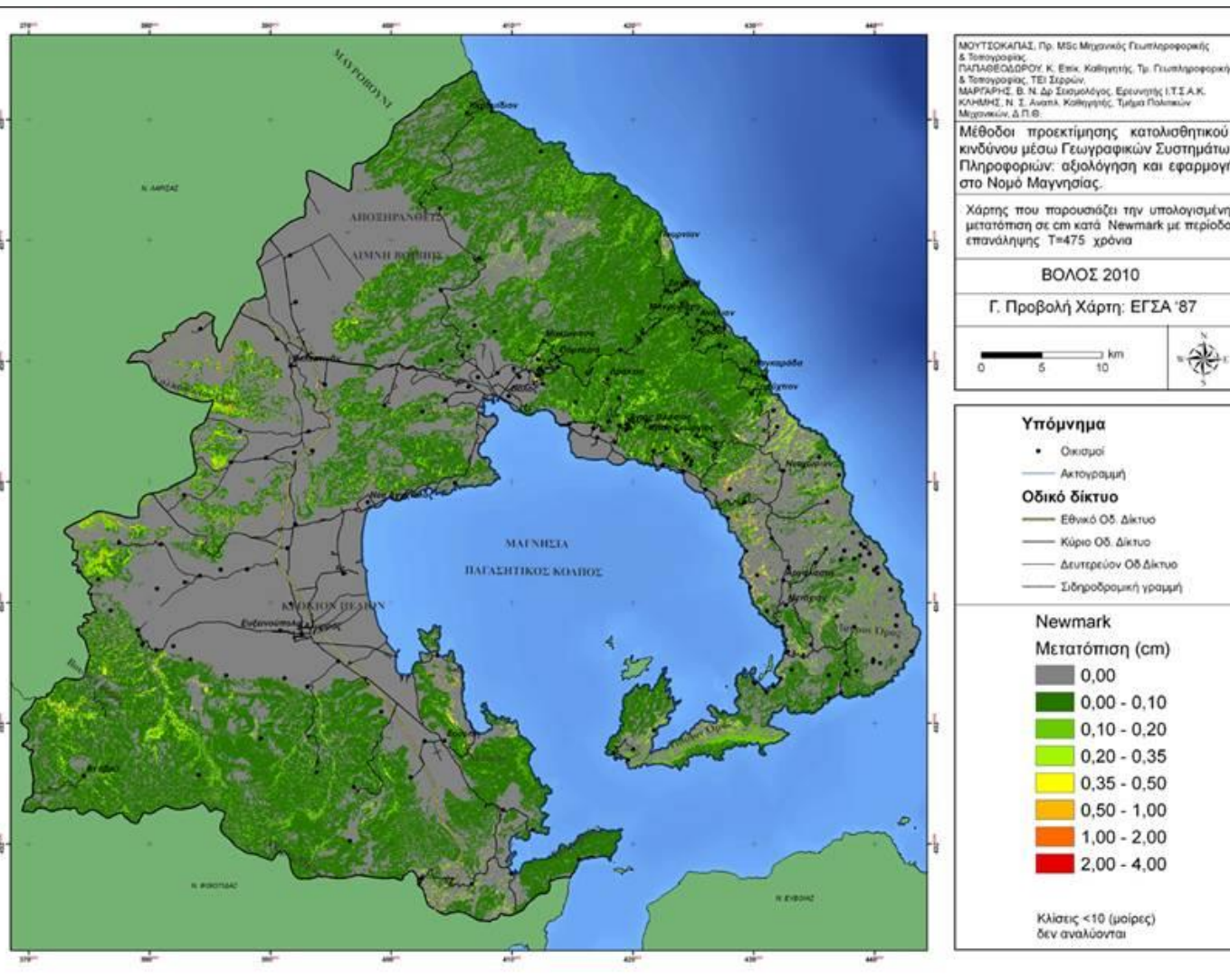
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Thematic Map

Scale: 1:50,000

Newmark displacements (cm)



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3. Landslide Hazard (static / hydraulic conditions) *according to the safety factor method (c)*

Factor of Safety Assessment:

$$FS = \frac{c'}{\gamma t \sin \alpha} + \frac{\tan \varphi'}{\tan \alpha} - \frac{m \gamma_w \tan \varphi'}{\gamma \tan \alpha}$$

ϕ' : effective angle of friction of geomaterial ($^{\circ}$)

c' : effective cohesion of geomaterial (kPa),

γ : specific weight (kN/m³),

α : slope angle (Deg),

γ_w : specific weight of th water (kN/m³),

t: normal thickness of the failure slab (m)

m: percentage of the water saturated failure slab (%)



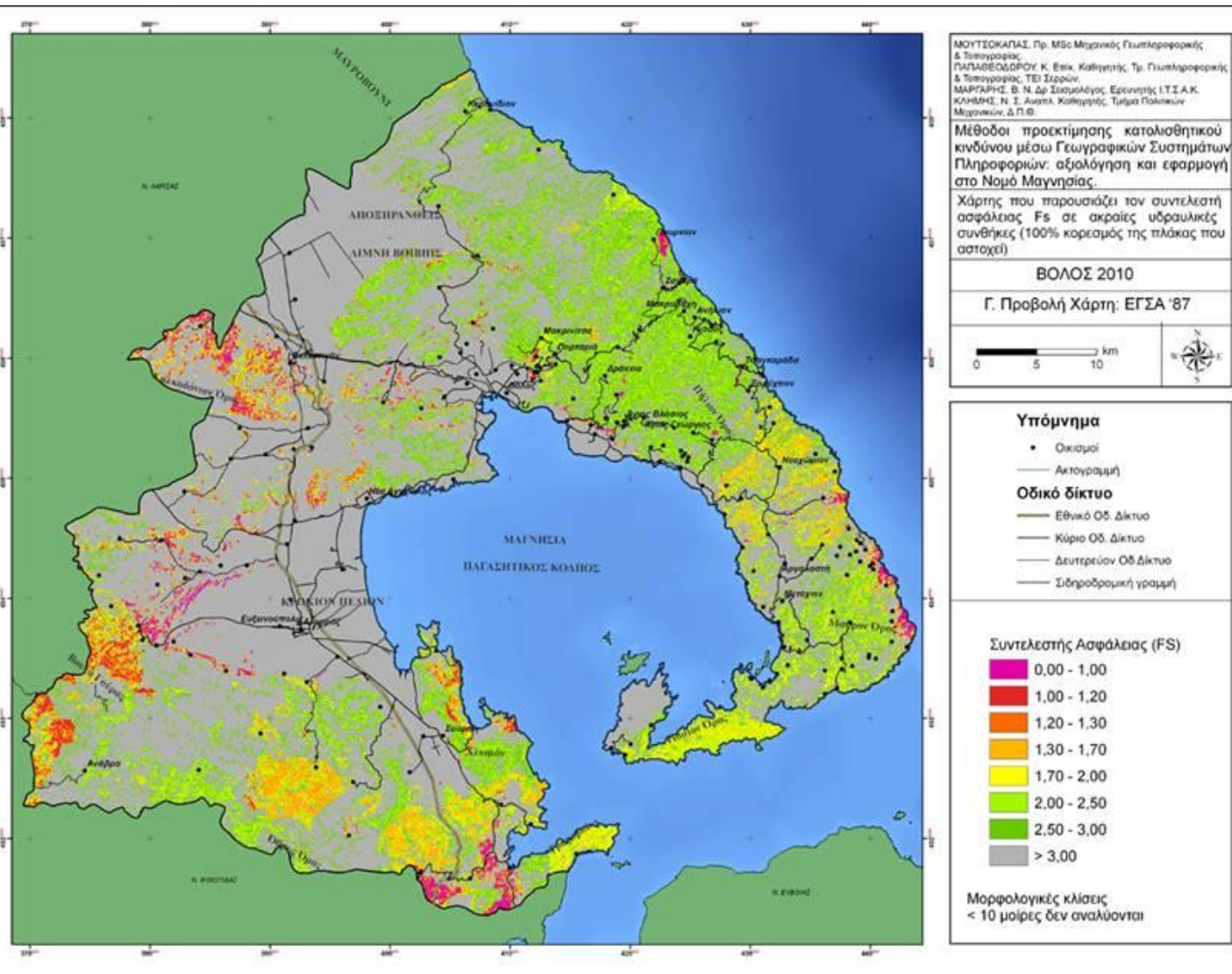
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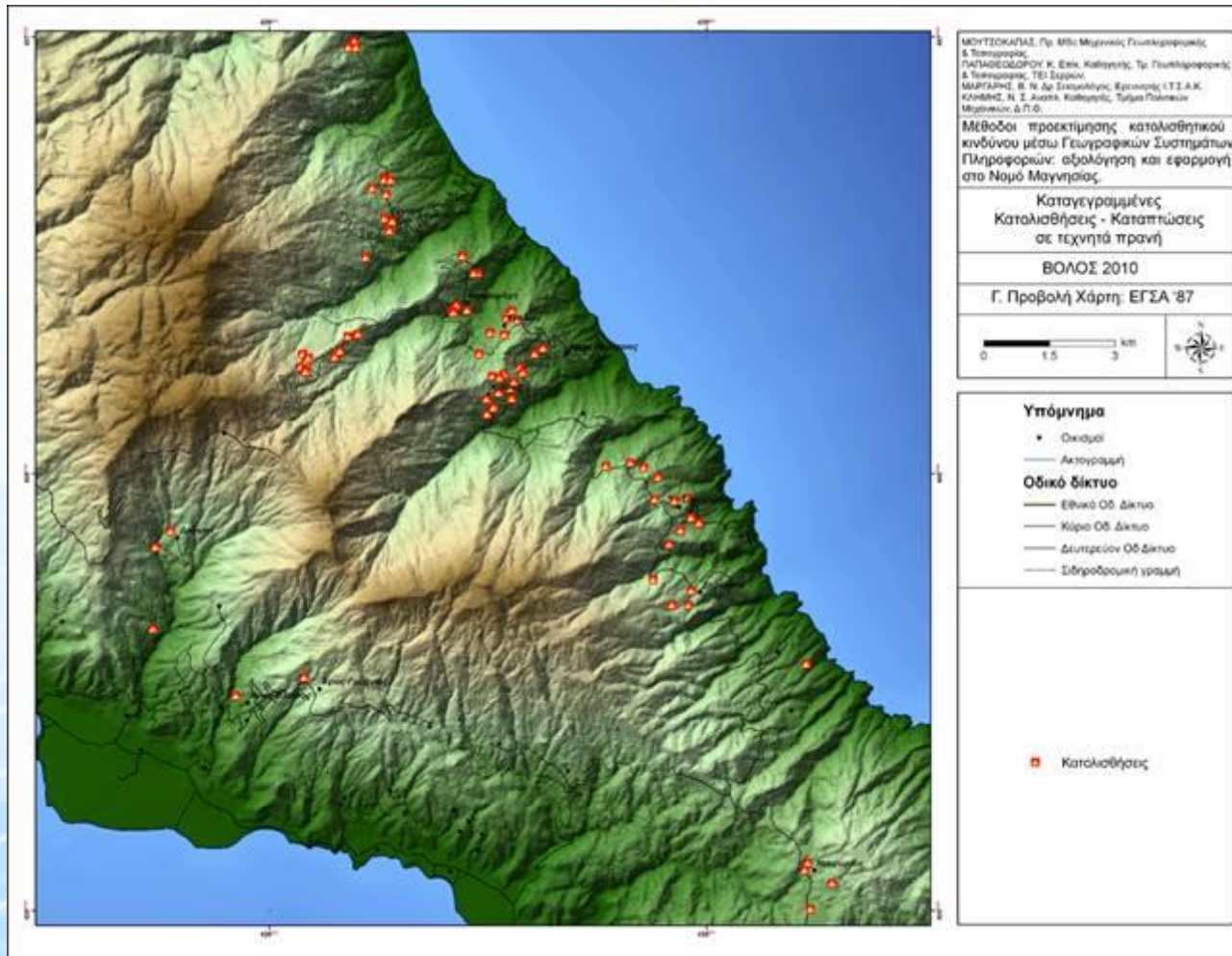
Thematic Map

Scale: 1:50,000

Safety Factor



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**Landslides on
cut slopes
(v:h=2:1)**

Scale: 1:50,000

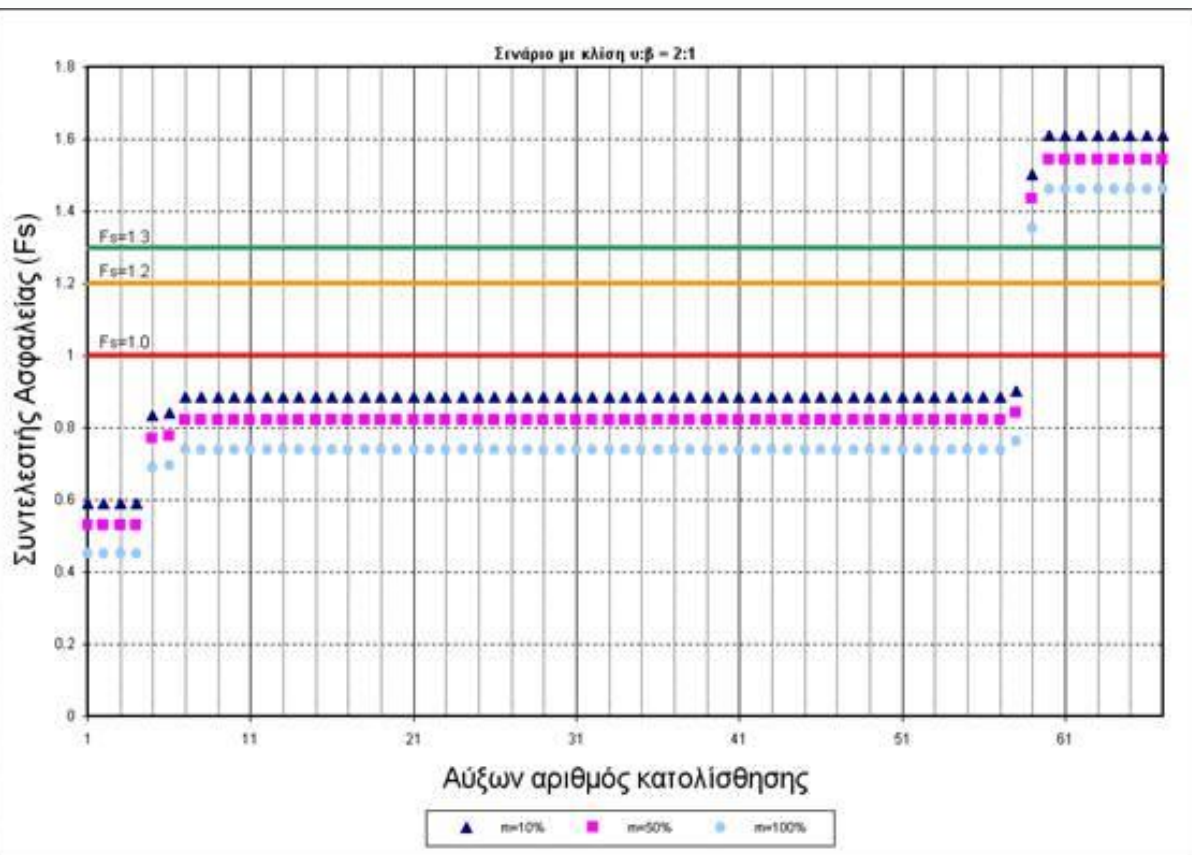
**Magnesia
Prefecture**

Landslide Hazard (static / hydraulic conditions) according to the safety factor method (c) in 67 locations where landslides occurred in cut slopes (vertical:horizontal = 2:1)

The above method, albeit crude, reached a percentage of almost 85% success.

Parametric investigation regarding saturation % of the sliding slab (m).

Slope-normal thickness of the failure slab (t) correlated to the slope angle (°).



Μορφολογική κλίση (°)	Πάχος πλάκας (t)
90° – 80°	t=0.0m
80° – 70°	t=1.0m
70° – 60°	t=1.5m
60° – 50°	t=2.0m
50° – 40°	t=2,5m
40° – 30°	t=4,0m
30° – 0°	t=10m

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Landslide Susceptibility Regional Scale 1:250,000 to 1:25,000 (*static & seismic conditions*):

1. **FEMA method (for static conditions: geologic maps + topography maps + hydraulic conditions) BUT needs improvement (introducing structure of soils/rocks: dip & dip direction of bedding, schistosity, interface of weathered zone and rockmass or soil over rockmass)**
2. **FEMA method (for seismic conditions: geologic maps + topography maps + hydraulic conditions) + Critical Acceleration: index A_c / PGA seems to work fine with local GMPEs and “shallow” landslides**

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Landslide Hazard Assessment Regional Scale 1:250,000 to 1:25,000 (static & seismic conditions):

1. **Factor of Safety method** (for **static conditions**: geologic maps + topography maps + hydraulic conditions (% of sliding slab saturation) + geotechnical parameters (ϕ' , c') + sliding slab normal thickness) seems to work fine for “shallow” landslides, **BUT** needs improvement (regarding assessment of sliding slab thickness)
2. **FEMA method** (for **seismic conditions**: geologic maps + topography maps + hydraulic conditions) + **Critical Acceleration**: index A_c / PGA resulting in assessment of **Permanent Ground Displacements** seems to work fine with local GMPEs and “shallow” landslides
3. **Modified Newmark method** (for **seismic conditions**: parameter of Arias Intensity **NOT easy to be obtained**)

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***Thank you so much for your
attention and patience***