





### Landslide Hazard Assessment on Regional Scales: **Pilot Implementation in Greece Acknowledgments:** SciNet NatHaz The SciNetNatHaz Project is partially funded by the EU and National funds within the context of the Black Sea Basin Joint Operational Programme 2007-2013

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### Landslide Hazard Assessment in the Blaxi area. The....Gaps!

- Usable Data are lacking. Inventories of past landslides do not exist or are not accessible.
- Metadata are not supplied so it's very difficult to assess reliability and accuracy of available data (if found!).
- Different LHA methodologies are used even in the same country, making comparison of outputs, impossible.
- Hazard identification & Risk assessment on regional and on local scales (that could provide the essential information for planning typical preventive measures) has only been sparsely implemented.







### SciNetNatHaz Landslide Hazard Assessment (LHA) related actions

The SciNetNatHaz project actions taken include:

- Select **worldwide accepted methodologies**, **applicable** in the wider area of the Black Sea basin, given the restrictions regarding data availability
- Adapt to "local" conditions and implement LHA on regional scales and in pilot implementation areas
- Compare selected methodologies in terms of feasibility to implement and accuracy & reliability of outputs
- Provide free access to data produced and processed with Metadata according to the INSPIRE directive provisions
- Indicatively assess the Risk on regional scale and implement analysis on a local scale which could provide the essential information for planning typical preventive measures.







### **Regional and Local scale LH Assessment to promote Prevention**



Landslide Hazard Assessment on Regional Scale to promote strategic planning & development

Hazard Identification Regional Scale  Stability analysis on a local scale to assess typical preventive measures







## Selected, Adapted to local conditions and Applied Methodologies

- A. Mora & Vahrson methodology (Sergio Mora C., & Wilhelm-Gunther Vahrson (1994): Macrozonation Methodology for Landslide Hazard determination. Bulletin of the Association of Engineering Geologists, Vol. XXXI No.1, 1994, pp.49-58.
- B. Federal Emergency Management Agency (FEMA, USA) methodology – HazUS (<u>https://www.fema.gov/hazus</u>)
- **C.** Factor of Safety calculation (Infinite Slope Model)







# **Implementation areas - Hellas**

Both areas of pilot implementation fall inside the Black Sea Programme eligible area:



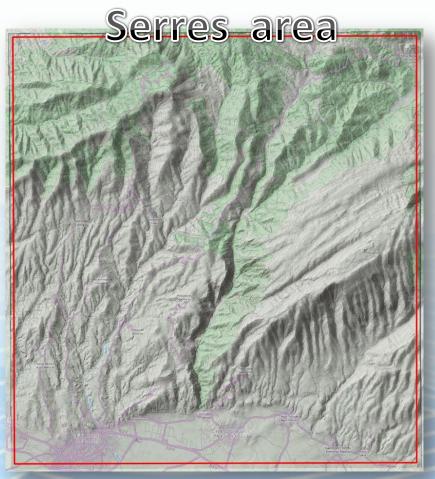
B. Komotini-Nymfaia







# **Implementation areas - Hellas**



## Komotini-Nymfaia\_area

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# A. Mora & Vahrson Methodology

Areas of pilot implementation

A. Serres

B. Komotini-Nymfaia

Scale of Regional Implementation

1:50.000 (input data & analysis)







# A. Mora & Vahrson Methodology

### **Calculates the "Intrinsic Landslide Susceptibility" (SUSC)** Taking into account the:

Slope Factor (**Sr**) Lithology Factor (**SI**) Soil Humidity Conditions (**Sh**)

And the Triggering Factor (TRIG) Deriver from the combination of Seismic factor (Ts) Precipitation factor (Tp) HI = SUSC \* TRIG= = (Sr \* SI \* Sh) \* (Ts + Tp)







# A. Mora & Vahrson Methodology

## The landslide Hazard indicator (HI)

# HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Where: Sr : "Slope" factor Sl : Geology factor Sh: Humidity factor

**Ts: (Earthquake) Seismic** triggering factor Tp: Precipitation triggering factor







# **A. M&V Methodology-Data requirements**

- Scale of Implementation 1:50.000
- Topographic data (topographic Maps, elevation data, lattice points etc). In case topographic data at a 1:50000 scale are not available, ASTER DEMs can be used at the expense of accuracy.
- Geologic Maps
- Ground Motion data (PGA values)

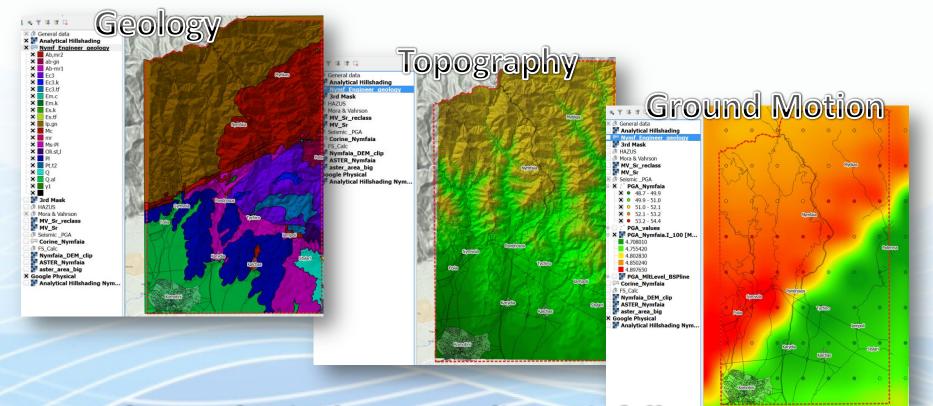
Mean Monthly Rainfall (mm) and MAX daily precipitations







# **A. M&V Methodology-Data requirements**



...and some basic data regarding rainfall







## **Susceptibility Indicator SUSC**

HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Includes the intrinsic properties of the landscape, the mechanical quality and its "passive" behavior. The parameters are the following:

- Slope Factor (Sr=Relative Relief),
- Lithology Factor (SI),
- Soil Humidity Conditions (Sh)







# Susceptibility Indicator SUSC – Slope Factor HI = SUSC \* TRIG = (Sr \* Sl \* Sh) \* (Ts + Tp)

The <u>Slope Factor</u> is defined by the maximum difference in elevation per unit area **Rr = Relative Relief per grid unit (square km) Rr =** (Hmax-Hmin)/km2 Relative Relief Classification Slope Factor Sr

Relative Relief Rr (m/km2)		Classification	Slope Factor Sr
	0-75	Very Low	0
	76-175	Low	1
	176-300	Moderate	2
	301-500	Medium	3
	501-800	High	4
	>800	Very High	5
	Table 1. Slope fac	tor classification	



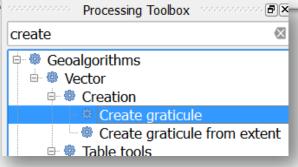




# **Relative Slope Factor(Sr) Calculation**

- **QGIS / Toolbox / Vector / Creation / Create graticule**
- SET as GRID EXTENT the DEM file in order to avoid

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- Grid Type: polygon
- Grid Extend: Select a layer which covers the implementation area (the DEM will be fine)
- Horizontal and Vertical spacing: 1000m
  - Grid CRS: Insert the **Geodetic Reference System** of your choice
  - Grid: grid name





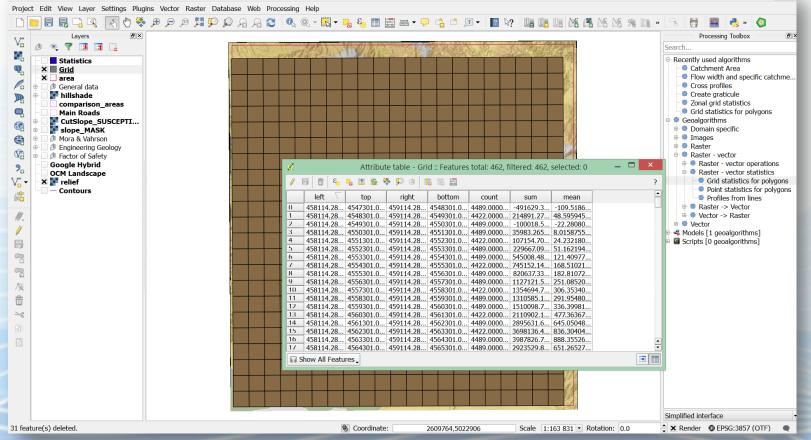


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Common borders. Common solutions.

# Relative Slope Factor(Sr) Calculation step 1/3

A Grid made of polygons, will be created (shapefile)



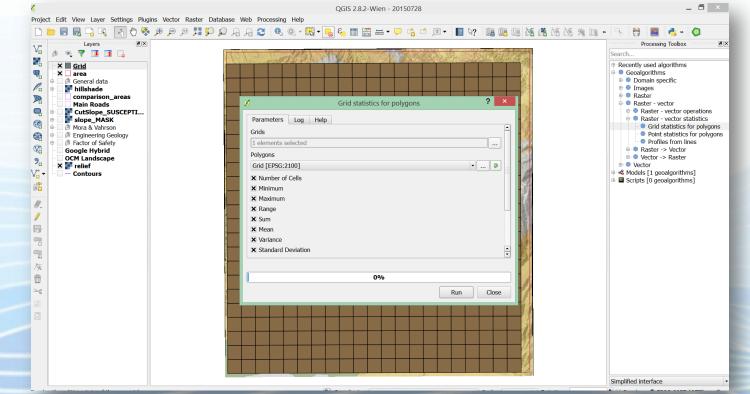






# Relative Slope Factor(Sr) Calculation step 2/3

- Toolbox/ Raster-Vector / Grid Statistics from polygons
- Insert the requested statistical parameters (especially "RANGE") and...Run









# Relative Slope Factor(Sr) Calculation step 2/3

A NEW shapefile will be created having as additional attributes the requested parameters including the "Range" which corresponds to the elevation difference in each 1km x 1 km cell.

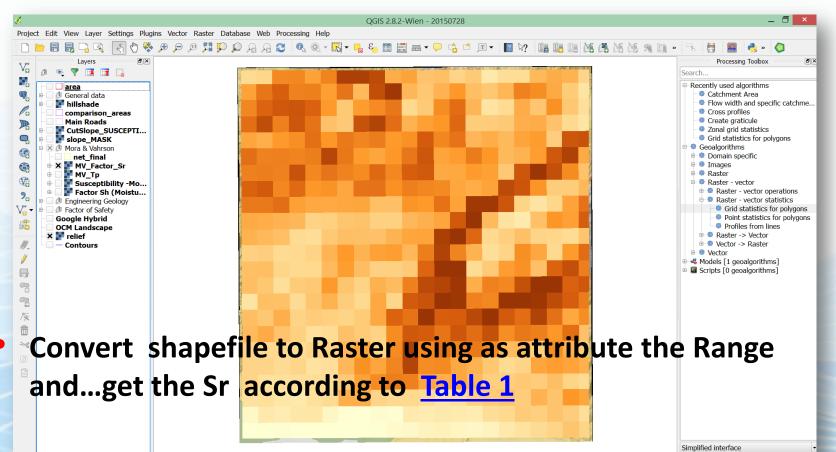
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# Relative Slope Factor(Sr) Calculation step 3/3









#### Common borders. Common solutions.

# **Relative Slope Factor(Sr) Calculation**

Layers	Kato Work
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# Susceptibility Indicator SUSC – Lithology Factor 1/2 HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Includes the intrinsic properties of the landscape, the mechanical quality and its "passive" behavior. The parameters are the following:

The Lithology Factor (SI), is assessed from the description of the geologic formations and ideally, geotechnical parameters should be taken into account. There should be an, as close as possible to real conditions, estimation of the geotechnical behavior of the geologic formations.







# Susceptibility Indicator SUSC – Lithology Factor 2/2 HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Parameters to be considered: volumetric weight, shear strength, weathering, discontinuities and their spatial distribution and orientation, their relation to slope geometry, drainage and pore pressure conditions, water table etc

**Lithology** can be used to **evaluate susceptibility** as very low (0), low (1), moderate (2), medium (3), high (4) and very high (5) (the highest the number the highest the susceptibility)



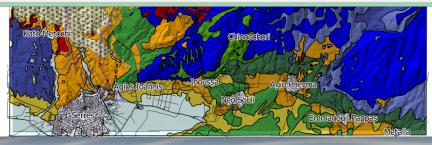




## Susceptibility Indicator SUSC – Geology Factor 2/2

Lithology factor according to Parameters considered (volumetric weight, shear strength, weathering etc)

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11	al	5339.410	349796.5	Ах	311	5	28	1.800	17.658	2.70000	5000.0000	17658.000	1887	5
12	ol	548.239	17449.036	Ах	321	32	19	2.300	22.563	1.50000	32000.0000	22563.000	0.3310	
13	ol	825.987	28115.140	Ах	321	32	19	2.300	22.563	1.50000	32000.0000	22563.000	0.3316	3
14	ol	2333.586	146100.5	Ах	321	32	19	2.300	22.563	1.50000	32000.0000	22563.000	0.3316	3
15	mr	2134.190	285691.6	Αχ	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
16	mr	644.204	23543.455	Аχ	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
17	mr	656.528	15992.729	Ах	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
18	Pt.t2	7594.766	727917.380	Ах	121	5	28	1.900	18.639	2.70000	5000.0000	18639.000	0.4887	4
19	mr	455.823	11108.927	Αχ	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
20	mr	620.850	18691.071	Αχ	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
21	mr	486.616	12868.357	Αχ	323	94	31	2.700	26.487	4.00000	94000.0000	26487.000	0.5411	0
22	el2	5093.986	812783.4	Ах	42	5	38	1.800	17.658	4.00000	5000.0000	17658.000	0.6632	5
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24	μν1	350.593	8441.049	Ах	412	117	46	2.650	25.997	3.00000	117000.00	25997.000	0.8029	1
25	sch1.gn	57651.583	2696933	- K	413	30	21	2.700	26.487	1.50000	30000.0000	26487.000	0.3665	3









### Susceptibility Indicator SUSC – Lithology Factor 2/2

Lithology factor according to Parameters considered (volumetric weight, shear strength, weathering etc)

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## **Susceptibility Indicator SUSC – Soil Moisture**

### HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Takes into account he average conditions of soil moisture. Quantifies the influence of **accumulated humidity throughout the year.** 

Best measured in situ, but usually a simple methodology of soil-water balance can be used requiring only the average monthly precipitations.

Steps to follow:

- 1. Calculate the AVERAGE Monthly potential evapotranspiration (PET) for the implementation area
- Each monthly average precipitation is assigned an index value according to Table 3
- 3. The TOTAL of all 12 month assigned values are calculated for each analyzed rain gage station. These values range from 0 to 24.
- 4. The total is classified into 5 groups according to Table 4







Average Monthly Precipitation AMP (mm/month)	Assigned Value	* 125 is the proposed value for South America. We must use a value around the average
<125*	0	monthly potential
126-250	1	evapotranspirationwhich may have been correlated to
>250	2	elevation.

Table 3. Average monthly rainfall values classification **Accumulated value** Qualification **Factor Sh** of Precipitation Indices 0-4 Very Low 1 5-9 2 Low 10-14 Medium 3 15-19 High 4 20-24 Very High 5

 Table 4. Moisture factor (Sh) from accumulated AMP values







## Triggering Indicator TRIG HI = SUSC \* TRIG = (Sr \* Sl \* Sh) \* (Ts + Tp)

Represents the EXTERNAL driving forces which trigger the event.

Combines two factors:

- i) the 100 year earthquake and
- ii) The 100 year intense rainfall events







<b>Triggering Indicator TRIG</b>	In ensitiet (MV) Tr=100yr	<b>Ke</b> Gualification	Factor Ts
HI = SUSC * TRIG = (Sr * SI * Sh) *	(Ts + Тр)	Slight	1
	IV	Very Low	2
Seismic intensity factor Ts is	V	Low	3
determined by analyzing	VI	Moderate	4
landslides triggered by	VII	Medium	5
earthquakes to assess the	VIII	Considerable	6
influence of seismic	IX	Important	7
intensities on the group of	Х	Strong	8
lithologic, climatic and	XI	Very Strong	9
morphologic conditions.	XII	Extremely Strong	10

Table 5. Seismic Intensity factor BASED ONOBSERVATIONS in Costa Rica and Central America







## **Triggering Indicator TRIG - Earthquakes**

HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

### The Seismic Intensity for 100yrs return period needs to be calculated







## **Triggering Indicator TRIG - Precipitation**

### HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

Maximum Rainfall n>10yrs, Tr = 100yrs	Rainfall n<10yrs; Average	Qualification	Тр Factor
<100mm	<50mm	Very Low	1
101-200mm	51-90mm	Low	2
201-300mm	91-130mm	Medium	3
301-400mm	131-175mm	High	4
>400mm	>175mm	Very High	5

**Table 6. Precipitation factor (Tp)** originating from the classification of maximum daily precipitations over a return period if 100yrs. An auxillary classification based on the average yearly maximum values per day is given in column 2.

Both Serres and Nymfaia areas present average maximum DAILY precipitations in the range of 51-90mm, so they fall into the **"Low"** category (Tp=2)







# **Classification of Landslide Hazard Indicator (HI)**

### HI = SUSC \* TRIG = (Sr \* SI \* Sh) \* (Ts + Tp)

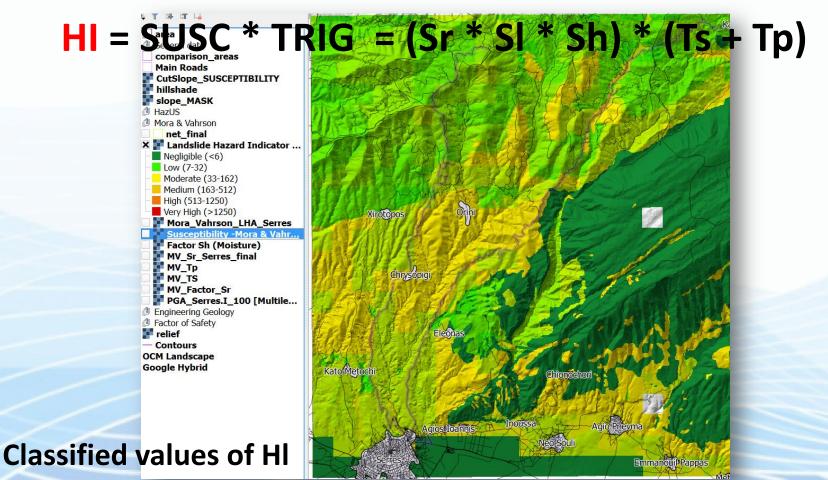
HI values range from 0 to 1250 or more	Value of HI	Class	Classification of Hazard of Landslide Potential				
	<6	I	Negligible				
These values are	7-32	II	Low				
indicative of the	33-162	III	Moderate				
landslide hazard	163-512	IV	Medium				
according to Table	513-1250	V	High				
7	>1250	VI	Very High				
	<b>Table 7</b> . Classification of the Landslide Hazard HI parametric values.						







## **Classification of Landslide Hazard Indicator (HI)**









END of the 1st cycle

# Areas of pilot implementation

A. Serres B. Komotini Viala (Constant)







# **B. FEMA methodology (Hazard US)**

Areas of pilot implementation

A. Serres

B. Komotini-Nymfaia

Scale of Regional Implementation

1:50.000 (input data & analysis)







# Landslide Hazard Assessment (FEMA)

- 1. Assess Landslide Susceptibility (under static conditions)
- 2. Assess the Critical Acceleration (Ac)
- **3.** Compare (Ac/PGA) the Critical Acceleration (Ac) with the actual Peak Ground Acceleration (PGA)

All the above parameters are calculated for two different moisture/groundwater conditions: "DRY" and "WET" whereas "DRY" corresponds to groundwater level BELOW surface of failure and "WET" corresponds to groundwater ON ground surface.







## **1.** Landslide Susceptibility under static conditions

Step 1: Geology (need to classify the formations in one of the 3 categories in Table 1)

Step 2: Slope angle (need to classify the formations in one of the six categories in Table 1)

Step 3: Underground Water Table (need to distinguish between "DRY" (when ground water below of level of sliding) and "WET" (when ground water at ground surface).







## Landslide Susceptibility under static conditions

#### Table 8. Landslide susceptibility under static conditions

(HazUS MH, Chapter 4 – PESH)

	Geologic Group		, SI	ope Ang	le, degre	es		
		0-10	10-15	15-20	20-30	30-40	>40	scale I: less susceptible
	(a) DRY (groundwate	r below l	evel of sli	ding)				coole Vi most sussentible
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 300 \text{ psf}, \phi' = 35^{\circ}$ )	None	None	Ι	п	IV	VI	scale X: most susceptible
в	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0, \phi' = 35^{\circ}$ )	None	ш	IV	v	VI	VII	None I I
с	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, c' =0 $\phi'$ = 20°)	v	VI	VII	IX	IX	IX	
	(b) WET (groundwater	level at	ground su	rface)				10
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 300 \text{ psf}$ , $\phi' = 35^{\circ}$ )	None	ш	VI	VII	VIII	VIII	
в	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0, \phi'$ = 35°)	v	VIII	IX	IX	IX	х	
с	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, c =0 $\phi$ = 20°)	VII	IX	x	X	x	x	



as shown on the map

e Current laver





#### **How to...**1/4 Geologic Group Common borders. Common solutions. (a) DRY (groundwate Strongly Cemented Rocks (crystalline rocks 1. Create a NEW column in "geology.shp" and classify the А and well-cemented sandstone, $c' = 300 \text{ psf}, \phi' = 35^{\circ}$ geologic formations according to **Table 8** in A, B & C Weakly Cemented Rocks and Soils (sandy в soils and poorly cemented sandstone, $c' = 0, \phi' = 35^{\circ}$ Categories Argillaceous Rocks (shales, clayey soil, slope\_10-15 С existing landslides, poorly compacted fills, c slope\_0-10 $=0 \phi' = 20^{\circ}$ 2. Create a NEW raster slope\_hazus 🗙 📑 C\_hazus C (c=0, f=20) (1) with all three B (c=0, f=35) (2) A (c>15kPa, f=35) (3) categories (as the C hazus A category 0.000000 1.000000 C\_Hazus shown) C hazus B category 0.000000 1.000000 C hazus C category 3. Create for **EACH** 0.000000 1.000000 Mora & Vahrson Orini Xirotopos category, a Engineering Geology DipDir Dip ALL NEW **SEPARATE** file to be Dip CLIP **TOBIA** classes **TOBIA** index Chrysopici used as MASK DipDir\_CLIP Eng\_Geology area\_GEOLOGY (C Hazus A category Factor of Safety Identify Results 8× etc) 🐛 🗈 🖨 Value Chianachar ture 4. The classes made will cover the entire area Agio Pney olos Ioannis

Auto open form

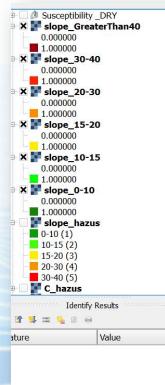


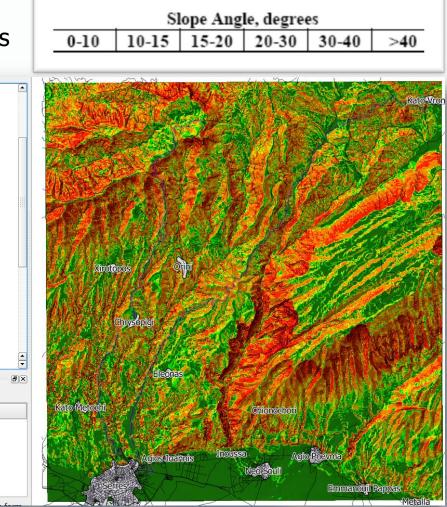




## How to...2/4 Common borders. Common solutions.

- Reclassify "slope" into the slope categories shown in *Table 8*
- 2. Create a NEW raster with all six categories (as the slope\_hazus shown)
- 3. Create for **EACH** category, a **SEPARATE file** to be used as **MASK** (slope\_0-10 etc)
- 4. The classes made will cover the entire area as shown on the map











Tahle 8

🗶 F slope\_GreaterThan40

0.000000 1.000000 Signature Signat

1.000000 Sippe\_20-30 0.000000

1.000000 Second States 15-20

0.000000 1.000000 September 10-15

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🗙 F slope\_0-10

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## How to...3/4

#### Common borders. Common solutions.

- Calculate the respective susceptibility category for each Geologic Group Class and then for each moisture condition (DRY or WET)
- i.e. For calculating the Geologic Group "A" susceptibility under WET conditions, the formula used was:

"C\_hazus\_A\_category@1" \* ( 0 \* "slope\_0-10@1" + 3 \* "slope\_10-15@1" + 6 \* "slope\_15-20@1" + 7 \* "slope\_20-30@1" + 8 \* "slope\_30-

20@1" + 7 \* "slope\_20-30@1" + 8 \* "slope\_30-40@1" + 8 \* "slope GreaterThan40@1" )\* See the layer names in the next image

Where:

- "C\_Hazus\_A\_category" is the mask to erase values outside this specific category area (is "1" where valid and "0" outside the specific category area)
- Slope\_\*@1 are the "slope" classes and
- the multiplication factors correspond to the susceptibility values shown in the table

	Geologic Group		S	lope Ang	le, degre	es	
		0-10	10-15	15-20	20-30	30-40	>40
	(a) DRY (groundwate	r below le	evel of sli	ding)			
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 300 \text{ psf}, \phi' = 35^{\circ}$ )	None	None	I	Π	IV	VI
в	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0, \phi' = 35^{\circ}$ )	None	Ш	IV	v	VI	VII
с	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, c' =0 $\phi' = 20^{\circ}$ )	v	VI	VII	IX	IX	IX
	(b) WET (groundwater	level at g	ground su	urface)			
A	Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, $c' = 3(0 \text{ psf}, \phi')$ = 35°)	None	Ш	VI	VII	VIII	VII
В	Weakly Cemented Rocks and Soils (sandy soils and poorly cemented sandstone, $c' = 0$ , $\phi' = 35^{\circ}$ )	v	VШ	IX	IX	IX	x
с	Argillaceous Rocks (shales, clayey soil, existing landslides, poorly compacted fills, c' =0 $\phi' = 20^{\circ}$ )	VII	IX	х	х	х	x







## **How to...**4/4 Common borders. Common solutions.

Susceptibility under different moisture conditions			<b>%</b>	Raster bands       WI values@1       Z_10m@1       Z_1m@1				Raster calculator Result layer Output layer Output format GeoTIFF			_susceptibility		
Geologic Group				igle, degree									
				20-30	30-40	>40	-	Curren	t layer extent				
(a) DRY (groundwater be         A       Strongly Cemented Rocks (crystalline rocks and well-cemented sandstone, c'=300 psf, φ'= 35°)       N	one	None	ng) I	п	IV	VI		X min Y min Column	615986.550 4552183.40 s 409	088 🗘 Y m	ax 627073.7 ax 4570650. vs 682		
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				Expression	valid						ОК	Ca	ancel







Emmanou

## Common borders. Common solutions. Landslide Susceptibility under static conditions

#### Susceptibility under different moisture conditions

is calculated by adding the individual Susceptibilities Per Geologic Group

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II 📕 🔤		
III		
IV V		
VI		
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VIII	i .	
IX		
X		
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	eol_Group_C_dry	
	e_GreaterThan40	
	e_30-40	
100 B 100	e_20-30	
	e_15-20	
slope	e_10-15 e_0-10	
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# Serres area DRY conditions Chrysopigi

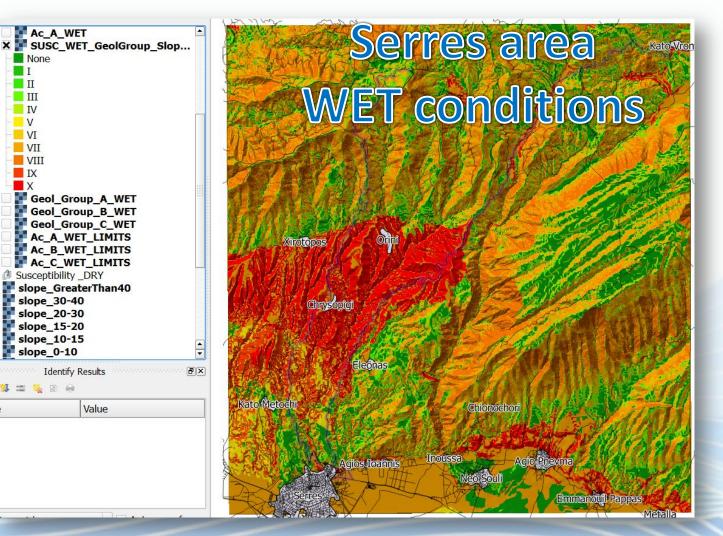






#### Susceptibility under different moisture conditions

is calculated by adding the individu Susceptibilities Per Geologic Group



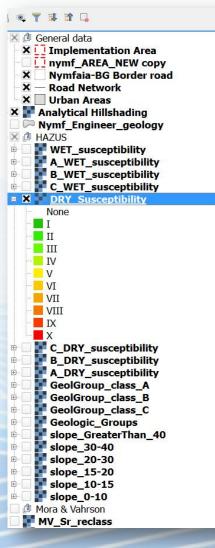


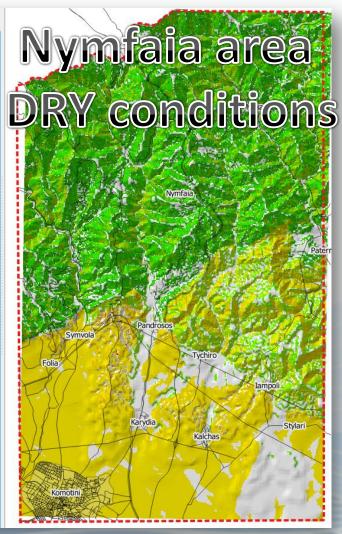




#### Susceptibility under different moisture conditions

is calculated by adding the individual Susceptibilities Per Geologic Group







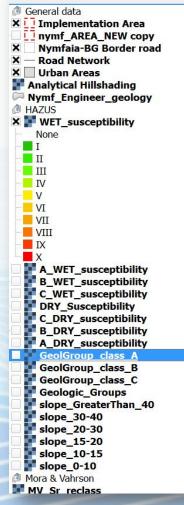




#### Susceptibility under different moisture conditions

is calculated by adding the individual Susceptibilities Per Geologic Group

#### s 🝸 💀 📬 🗔



## Nymfaia area **VET** conditions Nymfaia Pandrosos Symvola ychird Folia Iampoli Karydia Stylari Kalcha Komotini







## **2.** Landslide Susceptibility under seismic conditions

Limit Equilibrium Method principle: an earthquake is considered as a horizontal force (seismic coefficient \* weight of the potentially sliding mass of a slope)

**Critical Acceleration (A**<sub>c</sub>) is defined as the **horizontal** acceleration that produces a  $F_s = 1.0$ 

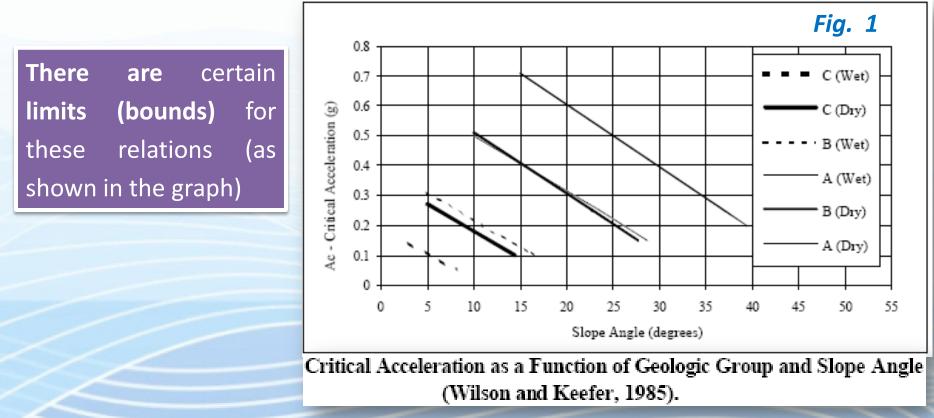






### Common borders. Common solutions. Calculating the Critical Acceleration A<sub>c</sub>

**Critical Acceleration (A<sub>c</sub>)** is a complex function of **slope**, **geology**, **steepness**, **groundwater table**, **type of landsliding** & **history of previous slope performance**.

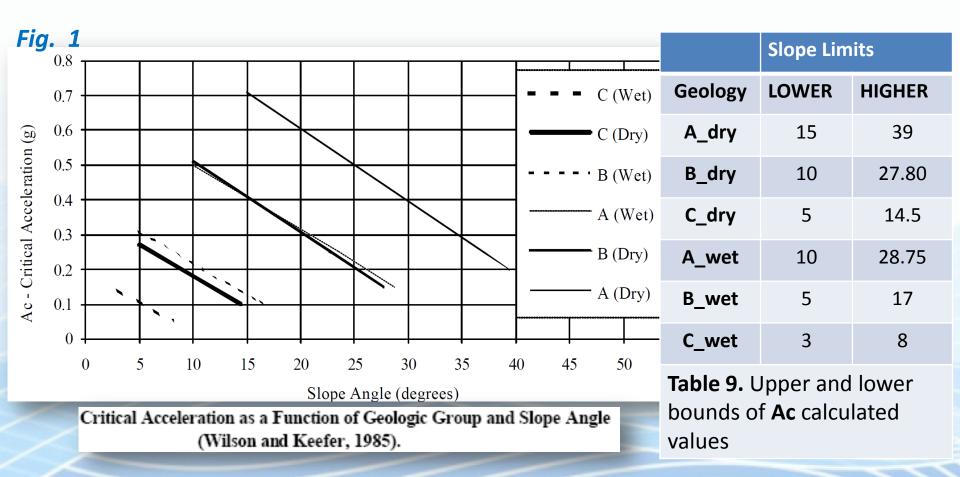








## **Slope limits for a valid Ac**









## Limits of slope angles and Critical Accelerations

#### Table 10

	Slope Ang	le, degrees	Critical Acceleration (g)			
Group	Dry Conditions	Wet Conditions	Dry Conditions	Wet Conditions		
Α	15	10	0.20	0.15		
В	10	5	0.15	0.10		
С	5	3	0.10	0.05		

 Table 11
 Critical Accelerations (ac) for Susceptibility Categories

[	Susceptibility Category	None	Ι	П	ш	IV	v	VI	VII	VIII	IX	х
	Critical Accelerations (g)	None	0.60	0.50	0.40	0.35	0.30	0.25	0.20	0.15	0.10	0.05





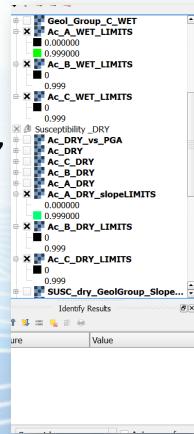


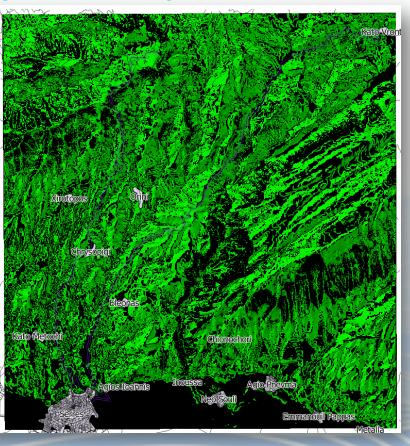
# Definition of Slope lower bounds for calculating a valid value of Ac (how to...)

To calculate the <u>Ac</u> <u>calculation slope based</u> <u>limits</u> (Table 2: 10-28 degrees) for the **Geologic Group "A" - WET conditions,** in QGIS Raster Calculator insert:

"slope\_CLIP@1" >= 10 AND "slope\_CLIP@1" <= 28

Where: "slope\_CLIP" is the slope map of the area in degrees





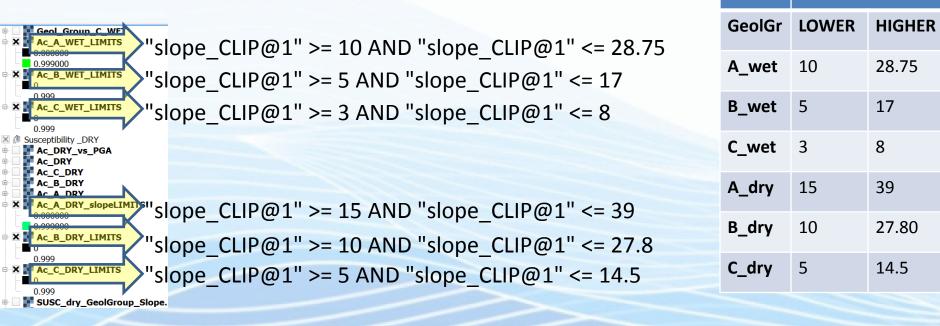






# Definition of Slope lower bounds for calculating a valid value of Ac (equations)

ie. To calculate the <u>Ac calculation slope based limits</u> (Table 2) for each Geologic Group and WET conditions, in QGIS Raster Calculator insert: Slope Limits









## Calculate the Ac for EACH geologic Group and moisture condition

0.8 need To that end, we the C (Wet) 0.7 equations for calculating Ac as a 0.6 50 C (Dry) Ac - Critical Acceleration 0.5 B (Wet) function of "Slope Angle", as 0.4 A (Wet) Wilson & Keefer shown in 0.3 B (Dry) 0.2 diagramm A (Dry) 0.1 . 0 0 5 10 15 20 25 30 35 45 50 55 Slope Angle (degrees) A dry A wet Bdry Cdry Bwet Cwet Slope -0.02094 -0.0186 -0.20469-0.01794 -0.01815 -0.01676 Interception 1.025017 0.68664 7.169656 0.397308 0.362313 0.187761 -0.99999-0.9999 -0.99998 -0.99974 -0.99997 -0.99903R

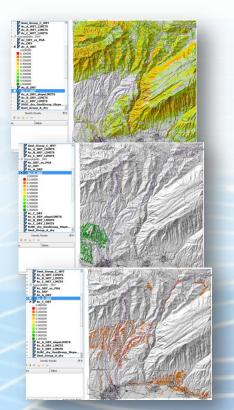






### Ac calculation for EACH geologic Group and moisture condition

Equations in RasterCalc : "Slope Based Upper and Lower Bounds" \* "Geologic Group Category" \* [equation for calculating Ac as a function of SLOPE]



"Ac\_A\_DRY\_slopeLIMITS@1" \* "C\_hazus\_A\_category@1" \* ( 1.025017 - 0.02094 \* "slope\_CLIP@1")

"Ac\_B\_DRY\_LIMITS@1" \* "C\_hazus\_B\_category@1" \* (7.169656 - 0.20469 \* "slope\_CLIP@1" )

"Ac\_C\_DRY\_LIMITS@1" \* "C\_hazus\_C\_category@1" \* ( 0.362313 - 0.01815 \* "slope\_CLIP@1" )

Respective equations are used for calculating the Ac limits for WET conditions







### **PGA values**

**PGA** values are calculated according to the seismic hazard assessment for the examined region (i.e. for 475 years or 1000years or 2000years) with the use of:

• either the relevant Ground Motion Prediction Equations (GMPE) suitable for the examined region and soil type

• or the relevant GMPEs suitable for the examined region and for rock conditions, which we will be multiplied with an amplification factor (PGA<sub>i</sub> =  $PGA_R * F_{Ai}$ )

Local GMPEs (Skarlatoudis et al., 2003) for the case of Greece

Example

 $logPGA = 1.07 + 0.45M - 1.35 \times ln(R + 6) + 0.09F + 0.06S \pm 0.286$ 

 $\log PGA = 0.86 + 0.45M - 1.27 \times \ln (R^2 + h^2)^{\frac{1}{2}} + 0.10F + 0.06S \pm 0.286$ 







## **Calculating PGA values**

 $PGA_i = PGA * F_{Ai}$ 

- $PGA_i$  is peak ground acceleration for site class i (in units of g)
- PGA is peak ground acceleration for site class B (in units of g)
- $F_{Ai}$  is the short period amplification factor for site class i, as specified for spectral acceleration  $S_{AS}(g)$

Site Class B			Site Class				
Spectral Acceleration	А	В	С	D	E		
Short-Period, S <sub>AS</sub> (g)	Short-Period Amplification Factor, FA						
≤ 0.25	0.8	1.0	1.2	1.6	2.5		
0.50	0.8	1.0	1.2	1.4	1.7		
0.75	0.8	1.0	1.1	1.2	1.2		
1.0	0.8	1.0	1.0	1.1	0.9		
≥ 1.25	0.8	1.0	1.0	1.0	0.8*		

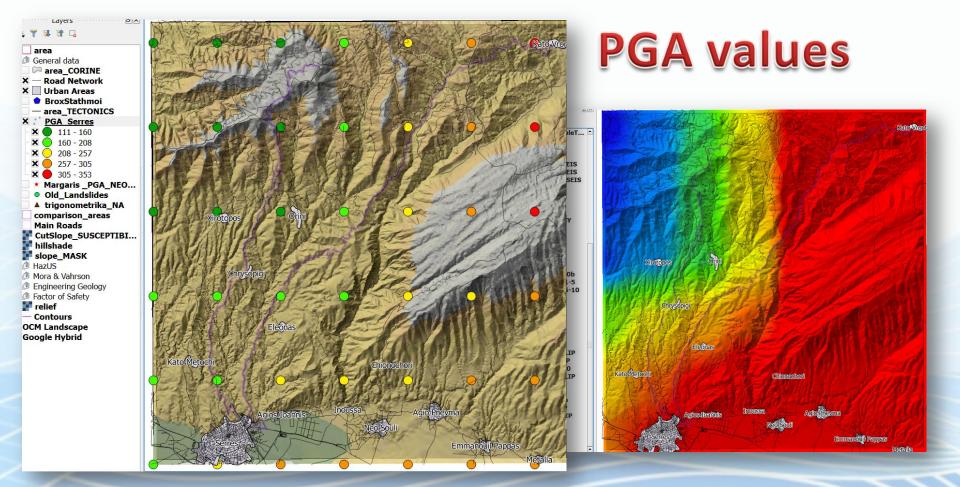
Soil amplification factors (Hazus 99-SR2 Technical Manual, Chapter 4-PESH)







## **PGA values for Serres pilot implementation area**



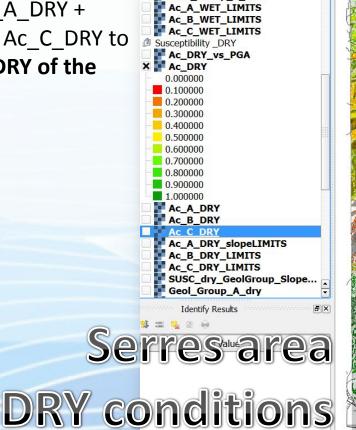






## Ac calculation for EACH geologic Group and moisture condition

ADD the Ac\_A\_DRY + Ac\_B\_DRY + Ac\_C\_DRY to get the **Ac\_DRY of the entire area** 



Geol Group C WET





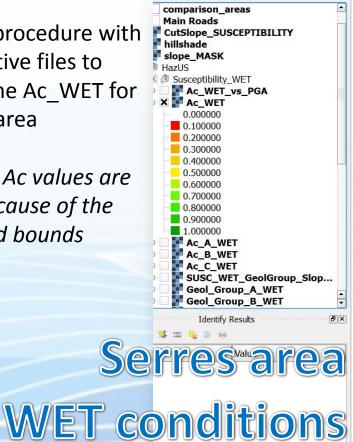


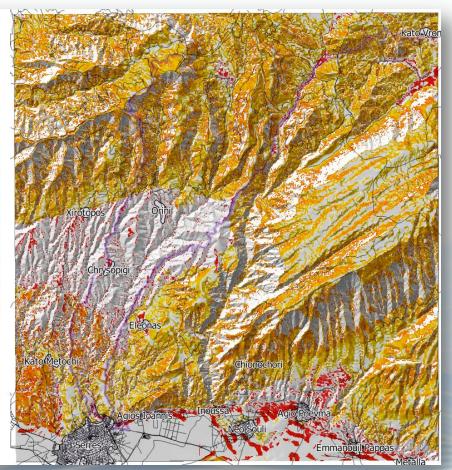


## Ac calculation for EACH geologic Group and moisture condition

The same procedure with the respective files to calculate the Ac\_WET for the entire area

Note: High Ac values are missing because of the slope based bounds applied











## "Shallow" landslide Susceptibility under seismic conditions

Critical Acceleration (A<sub>c</sub>) is defined as the horizontal

acceleration that produces a  $F_s = 1.0$ 

## **Criterion:**

Index A<sub>c</sub>/PGA and a "subjective" categorization

Very high: < 0.3</li>
High: 0.3 - 0.6
Moderate: 0.6 - 0.8
Low: 0.8 - 1.0
Very Low: 1.0 - 3.0
None: > 3.0

"Shallow" landslide susceptibility to earthquake-induced displacements, as specified by the index Ac/PGA





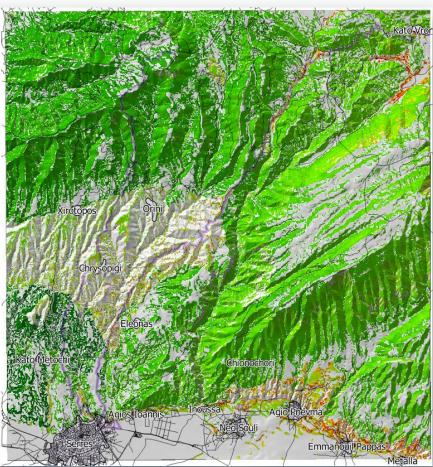


## "Shallow" landslide susceptibility under seismic conditions



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X Ac_DRY_vs_PGA	
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Very High (<0.3)	Contraction of the
High (0.3-0.6)	
Moderate (0.6-0.8)	CONTR.
Low (0.8-1.0)	1 NO 100 191
Very Low (1.0-3.0)	Server and Mar
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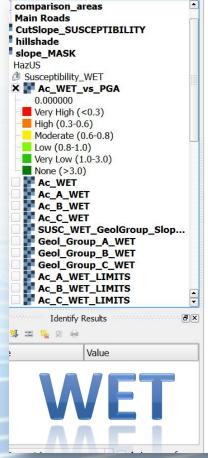


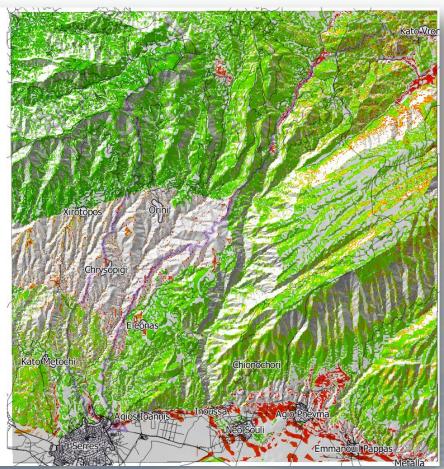




## "Shallow" landslide susceptibility under seismic conditions













## "Shallow" landslide susceptibility under seismic conditions

o 🍸 🗊 😭 🗔 🗇 General data

: F 3rd Mask HAZUS

0.000000

Ac WET

Ac\_DRY

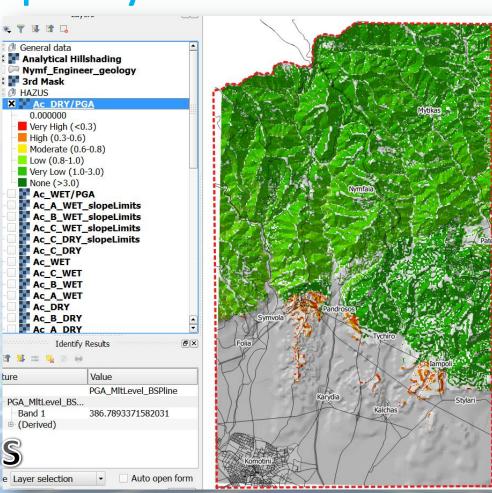
ture

Band 1

(Derived)



Nymfaia area DRY conditions



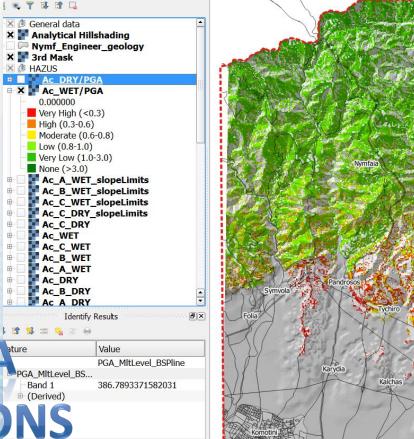






## "Shallow" landslide susceptibility under seismic conditions





#### Value PGA\_MItLevel\_BSPline PGA\_MItLevel\_BSPline PGA\_MItLevel\_BSPline PGA\_MItLevel\_BSPline PGA\_MItLevel\_BSPline (Derived) WETCONDITIONS ode Layer selection Auto open form







## **3.** Landslide Hazard under seismic conditions

- Based on FEMA method.
- Hazard is assessed over the calculation of Permanent Ground Displacements
- The methodology is applicable to Hazard assessment of "shallow" landslides

#### **Data Requirements**

Data produced during the previous implementation stages







## Permanent Ground Displacements (PGD) assessment

- The FEMA method is based on the assessment of PGD (Permanent Ground Displacements) for landslides;
- it is valid for "shallow" landslides, i.e with a depth of the failure surface not exceeding 7 to 10m max from the surface.

Requirements:

A<sub>c</sub>: critical acceleration (g); has already been analyzed and calculated in previous stages

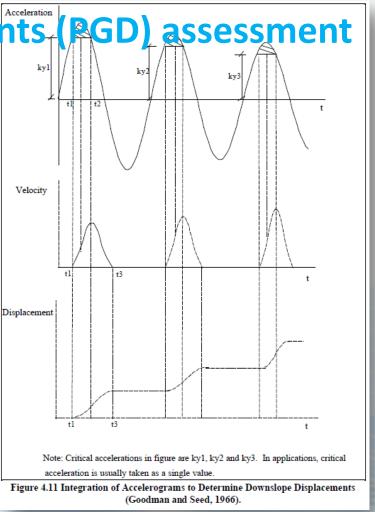






## Permanent Ground Displacements (PGD) assessment

The FEMA method is based on the assessment of PGD (Permanent Ground Displacements) for landslides (Goodman and Seed, 1966)









## Permanent Ground Displacements (PGD) assessment

# $E[PGD] = E[d/A_{is}]*A_{is}*n$

A<sub>is</sub>: induced acceleration (g) – A<sub>is</sub> = PGA
 A<sub>is</sub> = PGA : for shallow landslides
 A<sub>is</sub> = 2/3\*PGA: for massive, deep and large landslides

n: number of cycles (function of earthquake magnitude M<sub>w</sub>)
 E[d/A<sub>is</sub>]: expected displacement factor for each cycle







## n: Number of Cycles (moment Magnitude)

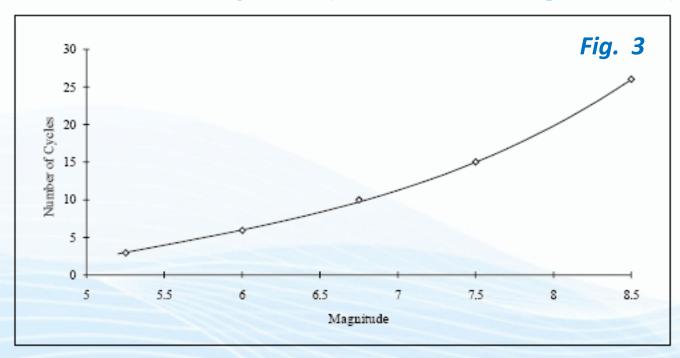


Figure 4.15 Relationship between Earthquake Moment Magnitude and Number of Cycles.

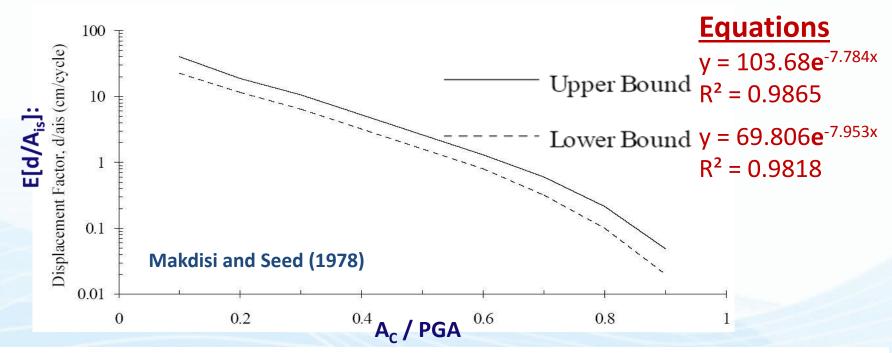
n = 0.3419M<sub>w</sub><sup>3</sup> – 5.5214M<sub>w</sub><sup>2</sup> + 33.6154M<sub>w</sub> – 70.7692 (Seed and Idriss, 1982)







## **Permanent Ground Displacements (PGD) assessment**



*Fig.2.* Relationship between Displacement Factor and ratio Critical Acceleration (Ac) to Induced Acceleration (PGA)

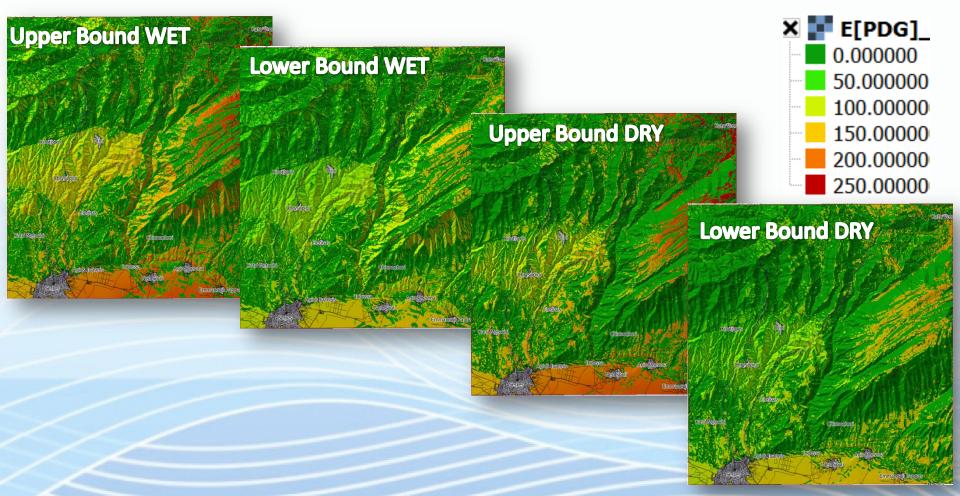
Calculation of the expected displacement factor in cm/cycle. Two (2) equations are given for the lower and upper bound for the earthquake induced permanent displacements







## Permanent Ground Displacements (PGD) assessment









# **B. FEMA methodology (Hazard US)**

C



B. Komotini-Nymfaia

# Scale of Region II be to the total of total of the total

1:50.000 (input data & a lysis)







## **C.** Landslide Hazard Assessment - FS

Areas of pilot implementation

A. Serres

B. Komotini-Nymfaia

Scale of Regional Implementation

1:50.000 (input data & analysis)







- Physically based landslide hazard assessment methods are based on *modeling of slope failure processes*
- Applicable over large areas if geological & geomorphological conditions are fairly homogeneous and landslide types relatively simple
- Applicable to areas with incomplete or even non existing landslide inventories
- Most of them apply the infinite slope model, therefore they are applicable in the case of shallow landslides
- Also, a deterministic model for plane or circular landslides can be applied
- They account for different triggering parameters: rainfall and transcient groundwater response or to the effects of earthquake excitation







# Landslide Hazard Assessment - FS

- Physically based landslide hazard assessment methods are based on modeling of slope failure processes
- the factor of safety F<sub>s</sub> computation method (triggering factors: rainfall & earthquake)







## Landslide Hazard– Static conditions / Precipitation

Infinite Slope Model (Factor of Safety)

$$F_{S} = \frac{c' + (\gamma_{app} - m * \gamma_{w}) * z * \cos^{2} \beta * \tan \varphi'}{\gamma_{app} * z * \sin \beta * \cos \beta}$$

Earth surface  

$$- \overline{z}$$
  
 $z_{w}$   
Failure surface  
 $m = z_{w} / z$   
 $z = Depth of failure surface (map ASHT)$   
 $z_{w} = Height of watertable$ 

$$\gamma_{\rm app} = \gamma * (1 - m) + \gamma_{\rm sat} * m$$

If totally dry slope, then  $\gamma_{app} = \gamma$  (m=0%)

If totally saturated slope, then  $\gamma_{app} = \gamma_{sat}$ (m=100%) φ': effective angle of friction of geomaterial (<sup>0</sup>)
c': effective cohesion of geomaterial (kPa),
γ: specific weight (kN/m<sup>3</sup>),
β: slope angle (Deg),
γ<sub>w</sub>: specific weight of the water (kN/m<sup>3</sup>),
z: normal thickness of the failure slab (m)
m: percentage of the water saturated failure slab (%)







# **C.** LHA Factor of Safety -Data requirements

- Scale of Implementation 1:50.000
- Topographic data (topographic Maps, elevation data, lattice points etc). In case topographic data at a 1:50.000 scale are not available, ASTER DEMs can be used at the expense of accuracy.
- Geologic Maps
- Engineering geologic/geotechnical parameters (Cohesion, Friction angle, unit weight)
- Ground Motion data (PGA values)
- Mean Monthly Rainfall (mm) and MAX daily precipitations

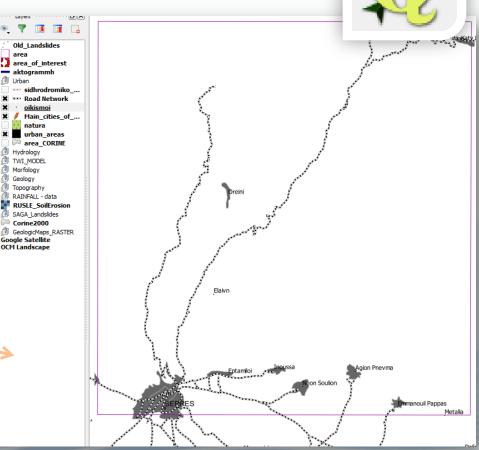






# **Basic Info – Starting a Project in**

- Set REFERENCE SYSTEM
- Input General data
  - Road network
  - Railroad network
  - Urban areas ....etc
- Define the AREA





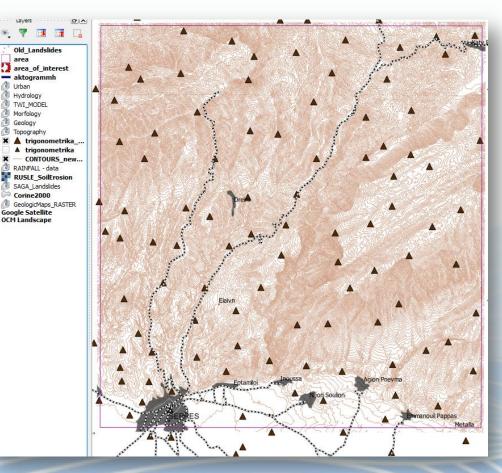




# **Topographic Data**

- Input topographic data
  - Contour Lines
  - Elevation points

 Please Note! We will be working on a 1:50.000 (Regional) scale



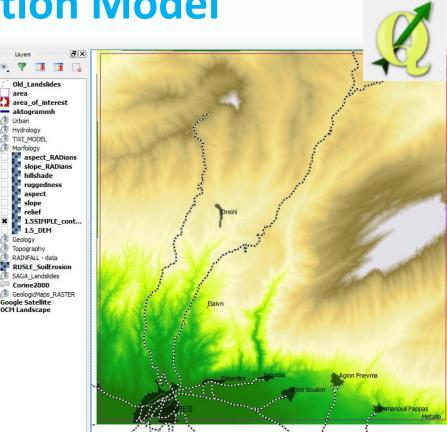






# **Digital Elevation Model**

- Create a Digital Elevation Model (DEM)
  - You can use your preferred method
  - ....BUT....
  - Pay attention to the PIXEL
     SIZE. Once defined it can not be changed and all outputs will be based on that. For a 1:50.000 scale map, contours per 20m, a pixel size of 15m is fine!
- In case the DEM covers a larger area, CROP it using the "AREA" polygon.





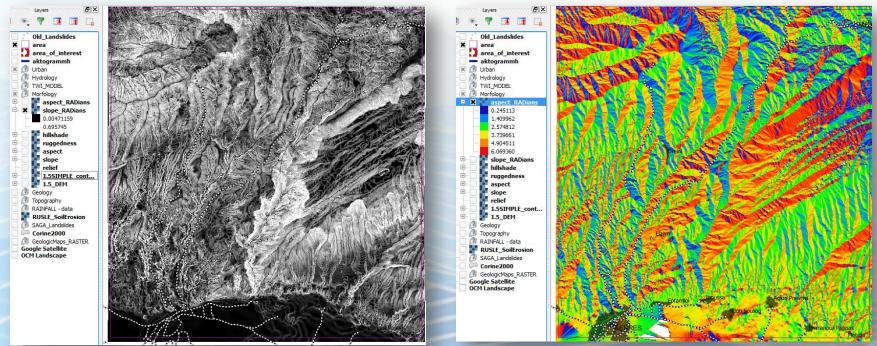




# **Slope and Aspect maps**



- Create SLOPE and ASPECT maps
  - Please Note! QGIS uses/calculates angles in RADIANS. Conversions in DEGREES may be needed in the process









### Calculating effective Cohesion (c') and friction angle ( $\phi$ ')

- IF the geological formation is a ROCKMASS, then Hoek and Brown failure criterion is used in order to establish two pairs of (φ' & c') for low and high normal stress (small slope and high slope).
- GSI (Geological Strength Index) and Uniaxial Compressive Strength must be estimated according to rockmass lithology and the condition of the rockmass.



RocLab 1.0







# Geotechnical parameters (c', $\phi'$ , $\gamma$ )

IF the geological formation is a SOIL, then according to the geological description (see geological maps) values of φ', c' and γ can be attributed according to international bibliography and your experience.

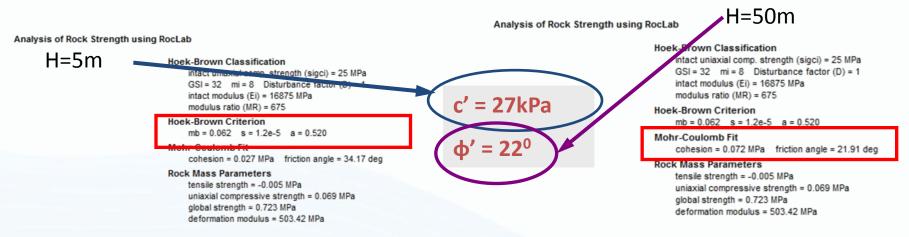
No matter if **SOIL** or **ROCKMASS** is encountered, geotechnical parameters should be estimated or calculated in a conservative way.





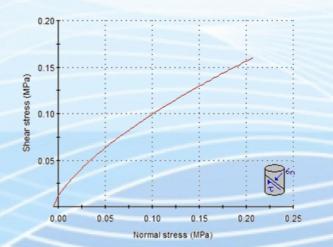


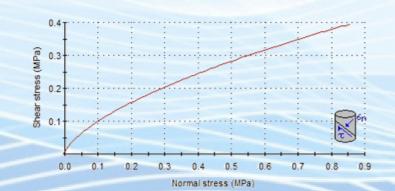
# Analysis of Rock Strengthussing RocLab



#### http://www.rocscience.com

#### http://roclab.software.informer.com/1.0/











### Calculating effective Cohesion (c') and friction angle ( $\phi'$ )

 Calculate i) Cohesion (c'); ii) angle of Internal Friction (φ); iii) Unit Weight and iv) Hydraulic Conductivity for each of the geologic formations of the area

	gn,ab			gn,ab,sch		gn,mr		gn-sch			gn-γ			gn-μν			gn1				
Hoek Brown Classification	H=5m	H=50m		H=5m	H=50m		H=5m	H=50m		H=5m	H=50m		H=5m	H=50m		H=5m	H=50m		H=5m	H=50m	
sigci (Mpa)	100	100		100	100		100	100		100	100		120	120		175	175		100	100	
GSI	30	30		31	31		30	30		29	29		31	31		31	31		33	33	
mi	23	23		23	23		12	12		10	10		26	26		26	26		25	25	
D	1	1		1	1		1	1		1	1		1	1		1	1		1	1	
Ei	40000	40000		30000	30000		85000	85000		30000	30000		48000	48000		70000	70000		30000	30000	
MR	400	400		300	300		850	850		300	300		400	400		400	400		300	300	
Hoek Brown Criterion																					
mb	0.15497	0.15497		0.16645	0.16645		0.08086	0.08086		0.06273	0.06273		0.18816	0.18816		0.18816	0.18816		0.20870	0.20870	
s	8.57E-06	8.57E-06		1.01E-05	1.01E-05		8.57E-06	8.57E-06		7.26E-06	7.26E-06		1.01E-05	1.01E-05		1.01E-05	1.01E-05		1.41E-05	1.41E-05	
a	0.52234	0.52234		0.52089	0.52089		0.52234	0.52234		0.52390	0.52390		0.52089	0.52089		0.52089	0.52089		0.51826	0.51826	
Failure Envelope Range																					
Application	Slopes	Slopes		Slopes	Slopes		Slopes	Slopes		Slopes	Slopes		Slopes	Slopes		Slopes	Slopes		Slopes	Slopes	
sig3max (Mpa)	0.12892	1.04787		0.12946	1.05227		0.12066	0.98074		0.12344	1.00331		0.13236	1.07584		0.13693	1.11300		0.13103	1.06504	
Unit Weight (MN/m3)	0.026	0.026		0.026	0.026		0.025	0.025		0.026	0.026		0.026	0.026		0.026	0.026		0.026	0.026	
Slope Height (m)	5	50		5	50		5	50		5	50		5	50		5	50		5	50	
Mohr-Coulomb Fit																					
c (Mpa)	0.0587	0.2299		0.0616	0.2391		0.0530	0.1798		0.0500	0.1653		0.0685	0.2671		0.0827	0.3084		0.0688	0.2651	
phi (degrees)	51.6	35.1		52.3	35.9		46.3	29.9		43.6	27.4		54.4	38.3		56.7	41.1		54.3	38.0	
Rock Mass Parameters																					
sigt (Mpa)	-0.0055	-0.0055		-0.0061	-0.0061		-0.0106	-0.0106		-0.0116	-0.0116		-0.0065	-0.0065		-0.0094	-0.0094		-0.0068	-0.0068	
sigc (Mpa)	0.2256	0.2256		0.2503	0.2503		0.2256	0.2256		0.2031	0.2031		0.3004	0.3004		0.4380	0.4380		0.3067	0.3067	
sigcm (Mpa)	4.5577	4.5577		4.7751	4.7751		3.2471	3.2471		2.8123	2.8123		6.1073	6.1073		8.9065	8.9065		5.4597	5.4597	
Erm (Mpa)	1128.98	1128.98		869.793	869.793		2399.08	2399.08		825.615	825.615		1391.67	1391.67		2029.52	2029.52		922.432	922.432	
Results																					
	H(m)	ф	c (kPa)	H(m)	ф	c (kPa)	H(m)	ф	c (kPa)	H(m)	φ	c (kPa)	H(m)	ф	c (kPa)	H(m)	φ	c (kPa)	H(m)	ф	c (kPa)
	5	51.61	58.71	5	52.30	61.65	5	46.31	52.98	5	43.63	50.02	5	54.43	68.54	5	56.70	82.66	5	54.27	68.77
	50	35.14	229.90	50	35.88	239.11	50	29.90	179.85	50	27.39	165.30	50	38.29	267.10	50	41.10	308.41	50	38.04	265.11
Final Values		ф	c (kPa)		ф	c (kPa)		ф	c (kPa)		ф	c (kPa)		ф	c (kPa)		ф	c (kPa)		ф	c (kPa
		35	59		36	62		30	53		27	50		38	69		41	83		38	69







### **Geotechnical Parameters Spatial Distribution**

- Digitize the Geologic Map
- Assign additional attributes to geologic formation polygons
  - C: effective Cohesion
  - Fi ( $\phi$ ): effective Internal Friction angle
  - Hc: hydraulic conductivity
  - ...etc....
- Please! Pay attention to the respective to each parameter, UNITS







# **Geotechnical Parameters Spatial Distribution**

Layers 🗗 🔀	Geo		Map											
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🗶 💋 gn-sch				IN CC	PERIMETER	AREA [GME_NAME]	DRO CO	C q		g_kN_m3 H	Ivd Conduc C.N	_m2 g.N_	n3 fi_RAD	tan.fi_R
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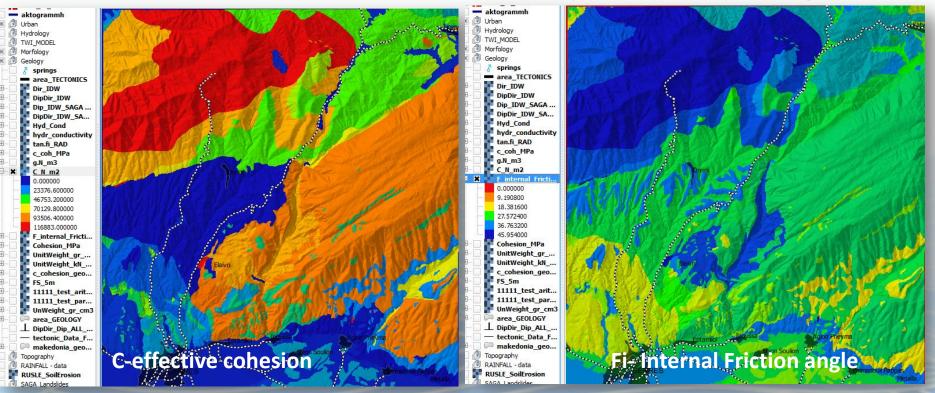






### Calculating effective Cohesion (c') and friction angle ( $\phi$ ')

 Preparing the parameters for the calculation of Factor of Safety (convert vector to Raster); c' – effective cohesion and φ'-Internal Friction angle



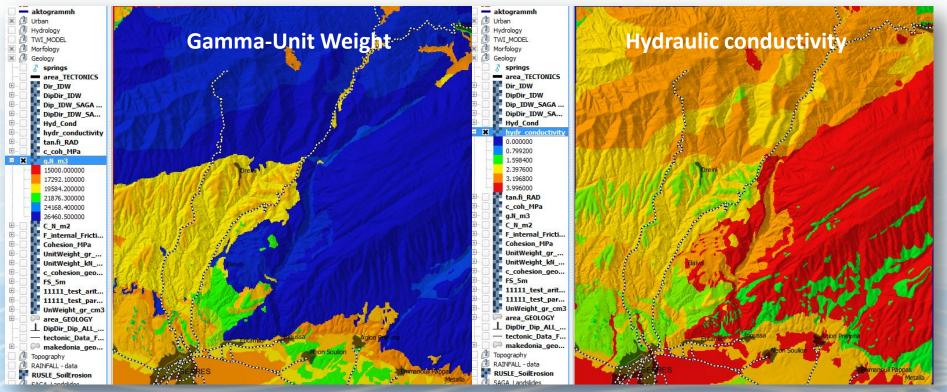






### **Unit Weight & Hydraulic Conductivity Spatial Distribution**

 Preparing the parameters for the calculation of Factor of Safety: Unit Weight and Hydraulic Conductivity (needed to calculate Saturation in SAGA GIS









# **Thickness and saturation of sliding slab**

The Most difficult parameters to estimate for the infinite slope model ! z: normal thickness of the failure slab (m) m: percentage of saturation of the failure slab (%)

- Normal thickness of failure slab (z) is to be determined as a function of slope angle (β), in order to calculate the factor of safety
- Percentage of saturation (m %) needs to be correlated with rainfall (mm) and a mean return period for the rainfall event (if such data exist for the examined region)







# **Thickness and saturation of sliding slab**

 The Normal thickness of failure slab (z) can be defined parametrically (i.e. 1m, 5m, 10m) and taken into account as such or physically based models can be used to link it to soil (and regolith) development natural parameters related to weathering, erosion and deposition (morphometric: slope, curvature, position on slope, etc; hydrologic, geologic etc)

**Indicative Relative Research:** Dietrich and Reiss, 1995; Catani et.al, 2010; Shafique et.al, 2011.

#### Potentially useful info:

- Pan-European Soil Databases fpr Landslide Mapping (JRC)
- <u>ESDAC Data Inventory</u>
- EU Soils







### Common borders. Common solutions. Saturation percentage of sliding slab

- Percentage of saturation (m %) needs to be correlated with rainfall (mm) and a mean return period for the rainfall event (if such data exist for the examined region)
- Create the Saturation Percentage (SP) using the WETNESS module in SAGA GIS
- Please note! The SP is calculated for a respective sliding mass thickness
- References and help are given within SAGA GIS (shown below).

#### **References:**

- Beven, K.J., Kirkby, M.J. (1979) A physically-based variable contributing area model of basin hydrology. Hydrology Science Bulletin, 24, 43-69..
- Montgomery D. R., Dietrich, W. E. (1994) A physically based model for the topographic control on shallow landsliding. Water Resources Research, 30, 1153-1171.

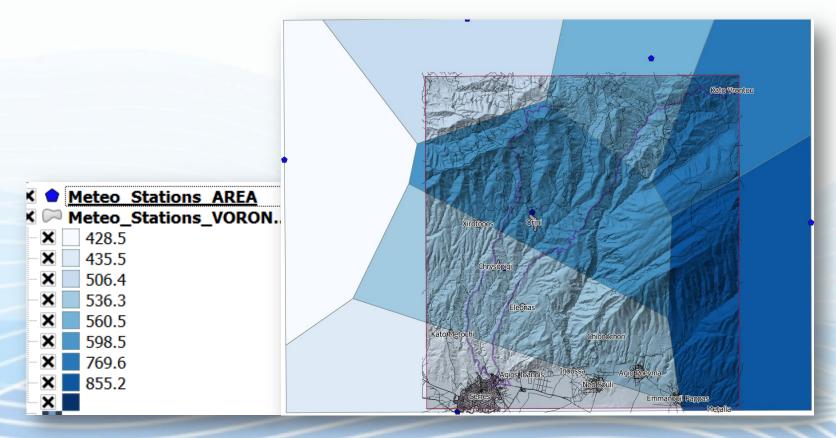






# Mean annual Rainfall (mm)

• Location of the Meteorological stations around the pilot implementation area





•





Common borders. Common solutions.

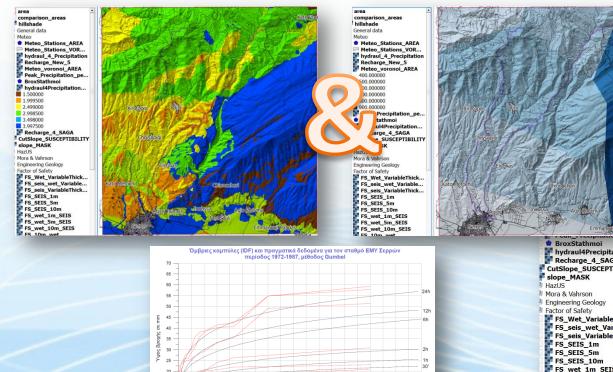
# SAGA "Recharge" (m/hour)

15

10

#### **Effective INFILTRATION**

#### **PEAK Rainfall** (m/hour)



http://users.auth.gr/vmarios/courses/IDF.pdf

**Recharge** (m/hour)

Agio Rnev

Emmanoull Pappas

Neo Soul

leona

os licennis

Kato Met

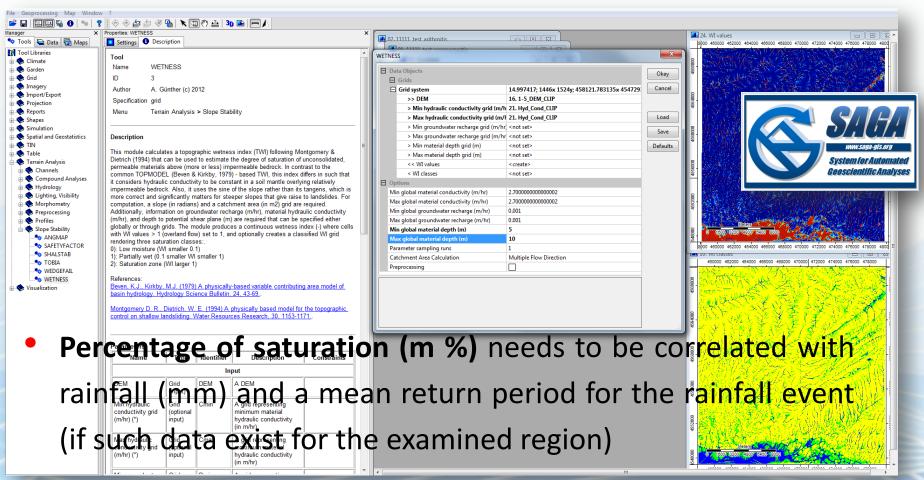








# Saturation percentage of sliding slab

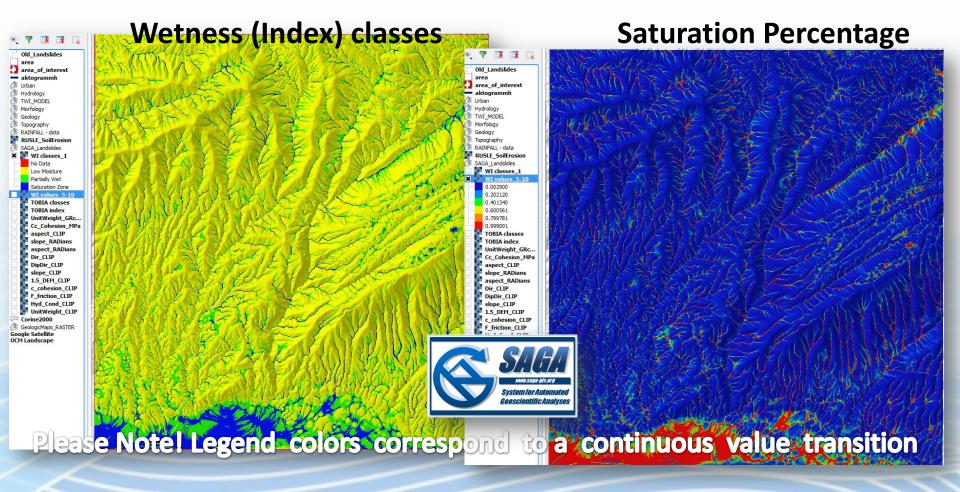








### Common borders. Common solutions. Saturation percentage of sliding slab







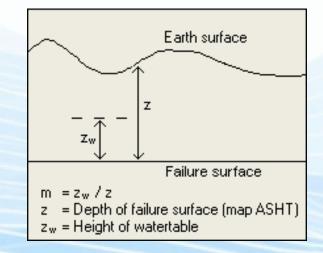


# Landslide Hazard – Seismic/Wet conditions

Infinite Slope Model (Factor of Safety for a Wet slope)

$$F = \frac{c' + (z\gamma\cos^2\beta - z\rho\alpha\cos\beta\sin\beta - \gamma_w z_w\cos^2\beta)}{z\gamma\sin\beta\cos\beta + z\rho\alpha\cos^2\beta}$$

φ': effective angle of friction of geomaterial (<sup>0</sup>) c': effective cohesion of geomaterial (kPa), β: slope angle (Deg), ρ: bulk density (Kg/m<sup>3</sup>) γ: specific weight (kN/m<sup>3</sup>),  $γ_w$ : specific weight of the water (kN/m<sup>3</sup>), a : earthquake acceleration (m/sec<sup>2</sup>) z: normal thickness of the failure slab (m) m =  $z_w/z$  % of the water saturated failure slab









# Landslide Hazard – Seismic/Wet conditions

Landslide Hazard is assessed for the following conditions

- **DRY:** Thickness of failure slab 1, 5 and 10m (three cases)
- WET: Thickness of failure slab 1, 5 and 10m (three cases)

### Seismic conditions

- DRY: Thickness of failure slab 1, 5 and 10m (three cases)
- WET: Thickness of failure slab 1, 5 and 10m (three cases)







### Calculate Factor of Safety– wet–5m thick sliding mass\*

$$F = \frac{c' + (\gamma - m\gamma_w) z \cos^2\beta \tan\phi'}{\gamma z \sin\beta \cos\beta}$$

in which:

c' = effective cohesion (Pa= 
$$N/m^2$$
).

- $\gamma$  = unit weight of soil (N/m<sup>3</sup>).
- m =  $z_w/z$  (dimensionless).
- $\gamma_{\rm w}$  = unit weight of water (N/m<sup>3</sup>).
- z = depth of failure surface below the surface (m).
- $z_w$  = height of watertable above failure surface (m).
- $\beta$  = slope surface inclination (°).
- $\phi$ <sup>•</sup> = effective angle of shearing resistance (°).

#### ...using the information layers created previously and the RASTER CALCULATOR module in QGIS

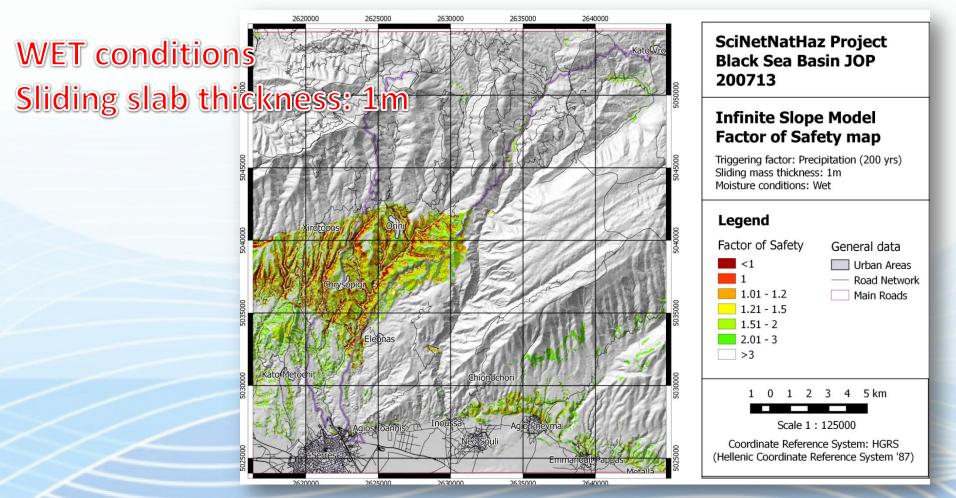
\* We are currently working into incorporating a geomorphological model to <u>calculate the soil thickness in the entire area</u>

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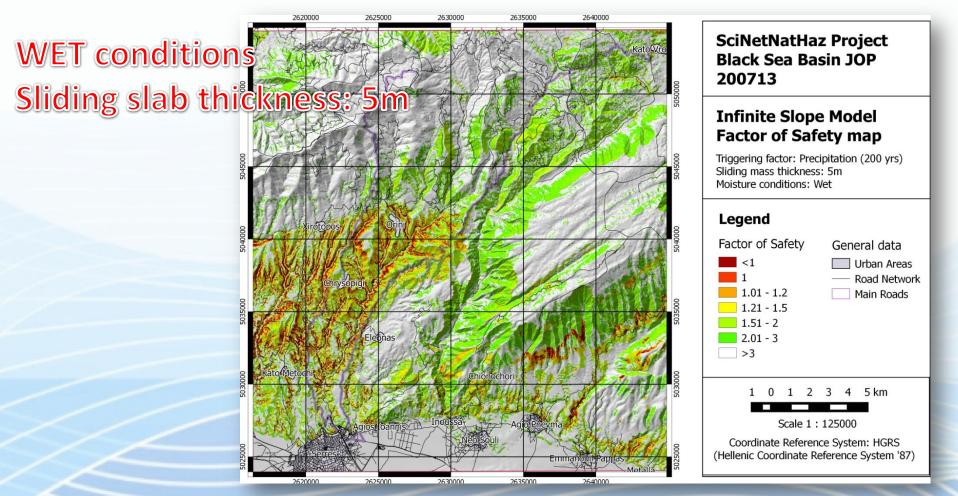








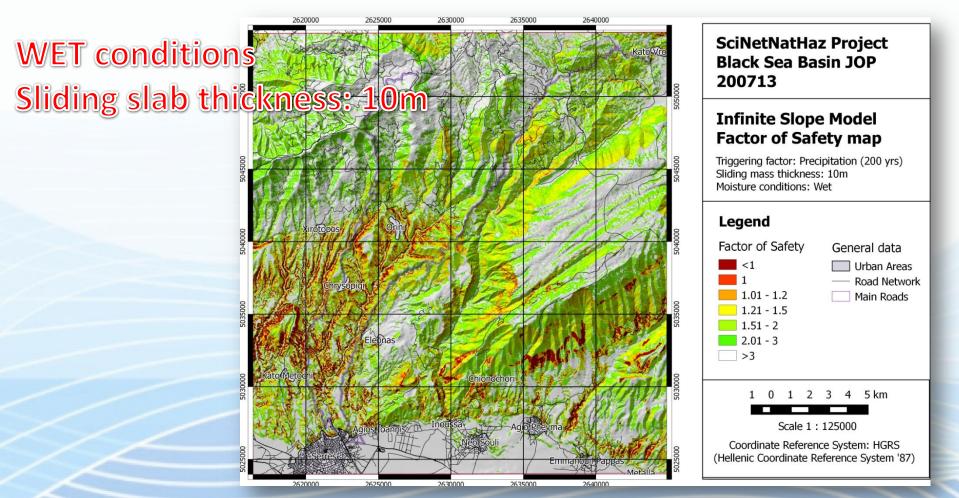








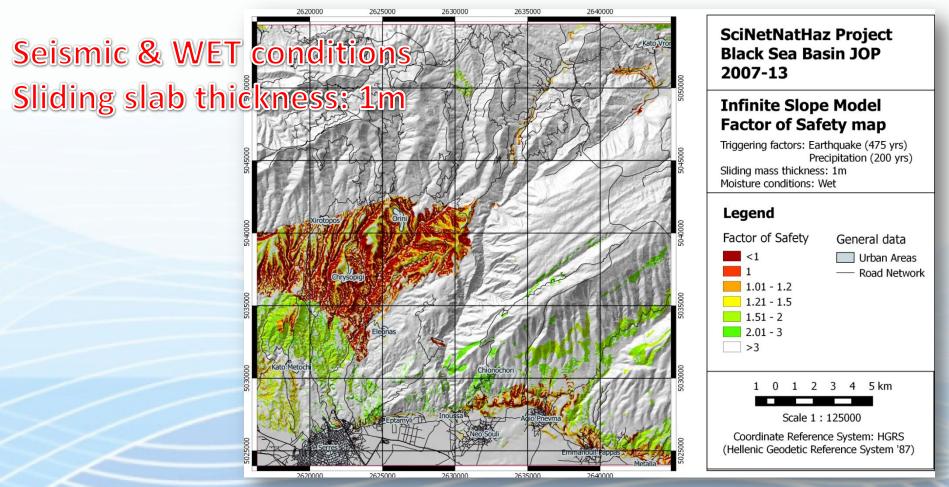










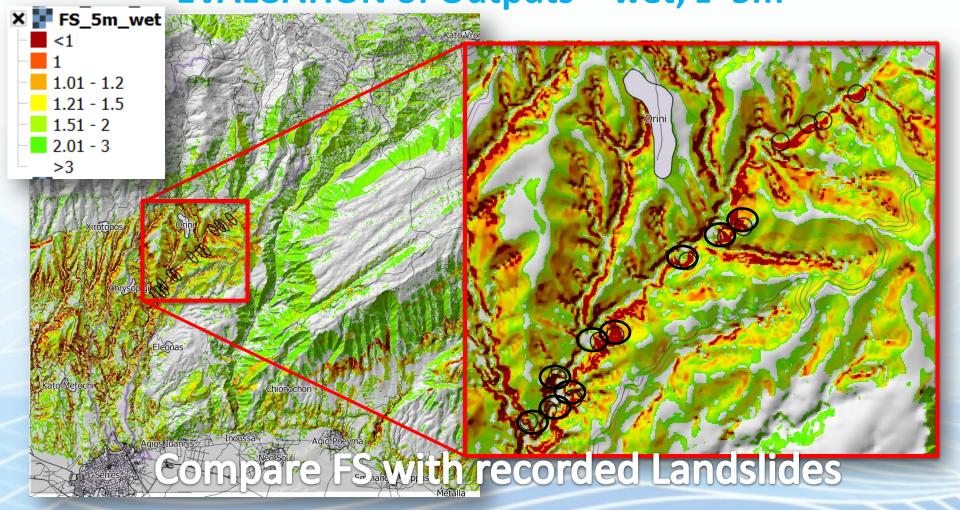








### Common borders. Common solutions. EVALUATION of Outputs – wet; z=5m

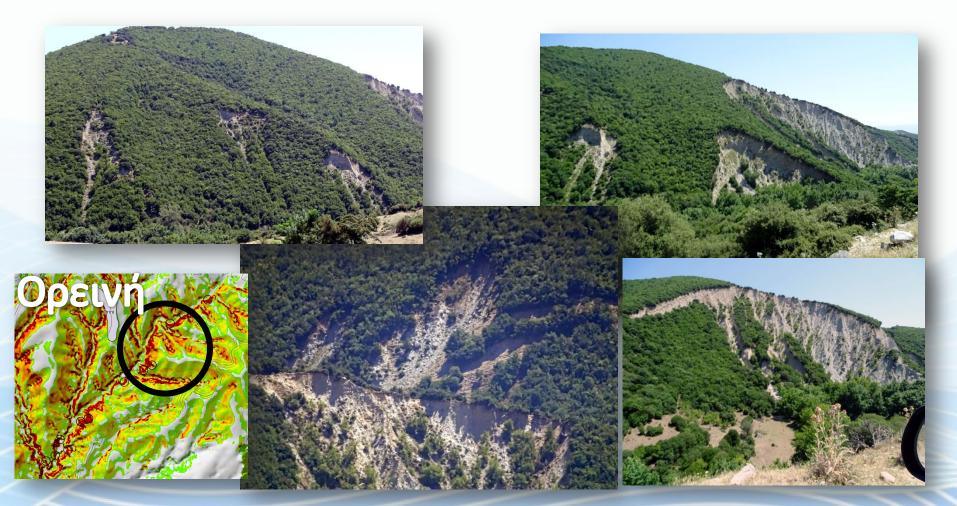








### Common borders. Common solutions. EVALUATION of Outputs – Recorded Landslides









## **General Conclusions (1/3)**

# Landslide Susceptibility Assessment, Regional Scale 1:250,000 to 1:25,000 (static & seismic conditions)

- FEMA method (for static conditions: geologic maps + topography maps + hydraulic conditions) needs improvements (introduction of structure of soils/rocks: dip & dip direction of bedding, schistosity, interface of weathered zone and rockmass or soil over rockmass)
- FEMA method (for seismic conditions: geologic maps + topography maps + hydraulic conditions) + Critical Acceleration Index [A<sub>C</sub>/PGA] seems to work fine with local GMPEs and "shallow" landslides







## General Conclusions (2/3)

### Landslide Hazard Assessment, Regional Scale 1:250,000 to 1:25,000 (static & seismic conditions)

- Factor of Safety method (for static & seismic conditions: geologic maps + topography maps + hydraulic conditions (% of sliding slab saturation) + geotechnical parameters (φ', c') + sliding slab normal thickness). For seismic conditions bulk density (ρ) + earthquake acceleration (a) are needed. The methodology works fine for "shallow" landslides BUT needs some improvement regarding the assessment of sliding slab thickness.
- FEMA method (for seismic conditions: geologic maps + topography maps + hydraulic conditions) + Critical Acceleration Index [A<sub>c</sub>/PGA] resulting in the assessment of Permanent Ground Displacements, seems to work fine with local GMPEs and for "shallow type" landslides.







### Landslide Hazard Assessment Scales: YOUA Aank Greece SciNe NatHaz Acknowledgments: NatHaz Project ir Attentio National funds within the context of the Black Sea Basin Joint Operational Programme 2007-2013

K. Papatheodorou, TEI of Kentriki Makedonia, Hellas Nikolaos Klimis, Democritus University of Thrace

SciNetNatHaz project Open Seminars, September-October 2015



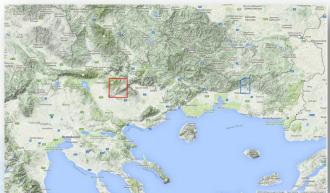




# Landslide Susceptibility -Some additional Info

Landslide susceptibility is closely related to low shear strength surfaces as :

- Bedding
- Schistocity
- Joints



Recording of the later (joints), requires field work thus...local scale implementation (costs in time and money)

Information regarding the other two parameters, can be digitized from geologic maps (minimal cost)







# Landslide Susceptibility Mapping

Create TOBIA index and Classes..



- ..Using the respective SAGA GIS module and..
- ...Slope, Aspect, Dip and DipDirection maps

Meentemeyer R. K., Moody A. (2000). Automated mapping of conformity between topographic and geological surfaces. Computers & Geosciences, 26, 815 - 829.

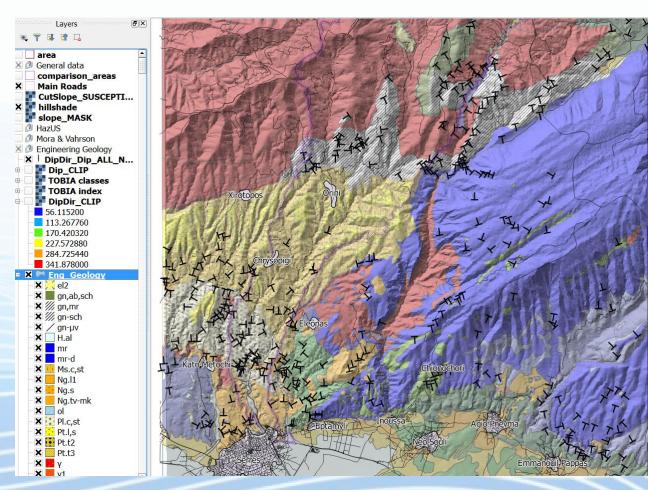






# Landslide Susceptibility Mapping

Digitize Bedding and Schistocity orientation (Dip & Dip Direction) from geologic maps



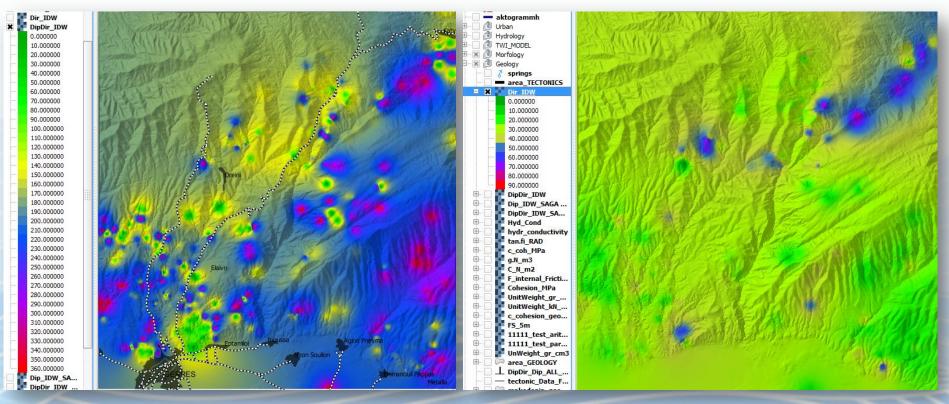






# Landslide Susceptibility Mapping

• Create spatial distribution maps of each parameter using the IDW method (does not exceed minimum-maximum value limits)

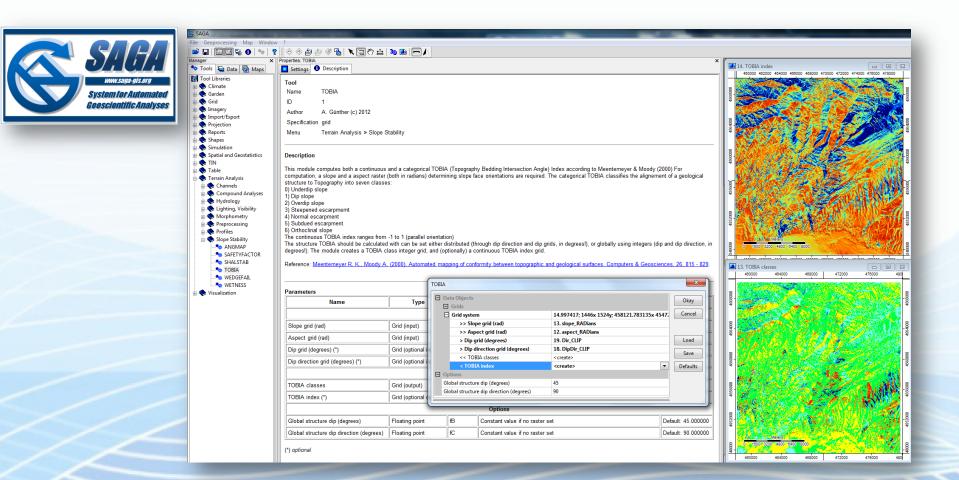








# Use SAGA "TOBIA" module to ....

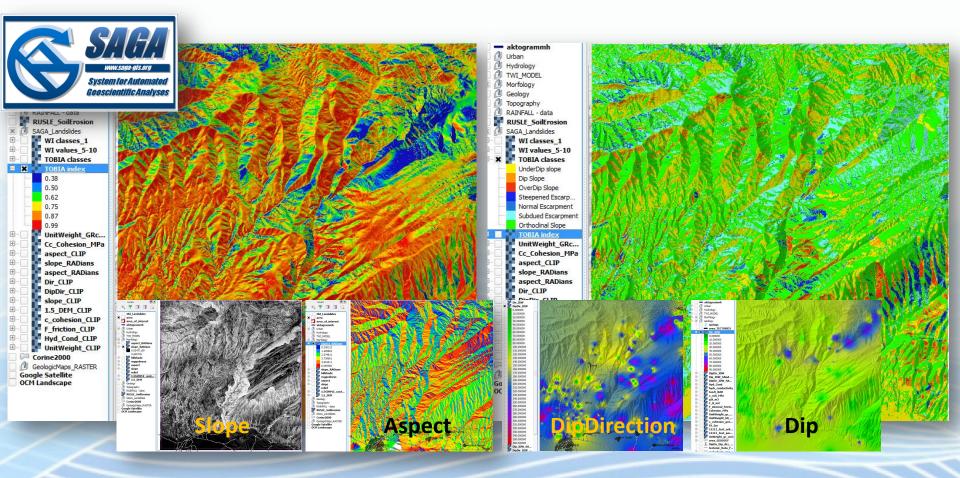








### ..compare Slope with Dip and Aspect with DipDirection..

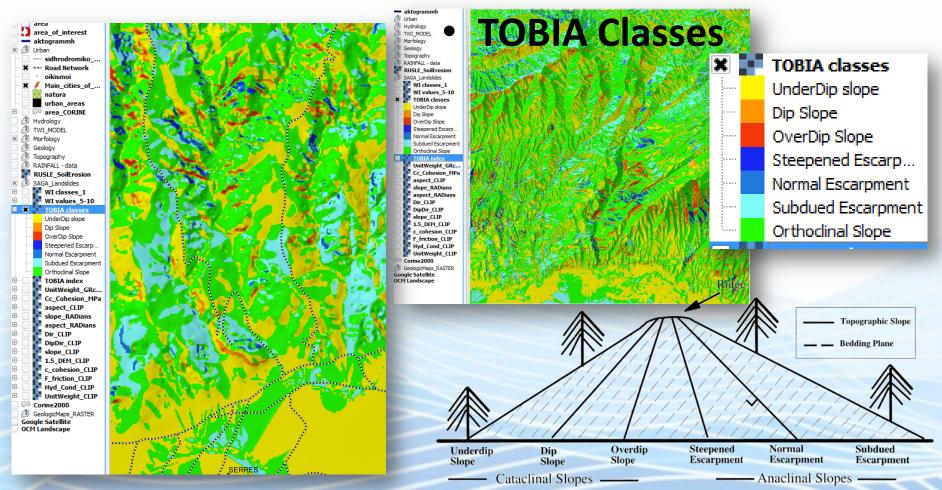








### ..and create TOBIA Index classes..









### Landslide Hazard Assessment al Scales: <u>legion</u> aan Greece SciNe NatHaz Acknewledgments: Attentio el NatHaz Project enc National funds within the context of the Black Sea Basin Joint Operational Programme 2007-2013

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